

DESIGN PHILOSOPHY

THIS DRAINAGE DESIGN HAS BEEN PUT TOGETHER TO DISCHARGE THE BELOW CONDITIONS ISSUED BY HORSHAM DISTRICT COUNCIL:

2 - Regulatory Condition: Within three months of the date of this decision, a drainage strategy detailing the proposed means of foul water disposal shall be submitted to the Local Planning Authority for its approval in writing. The drainage strategy shall be implemented within 3 months of such approval being given in strict accordance with the agreed details. The drainage strategy shall be retained as such thereafter.
Reason: As this matter is fundamental to ensure that the development is properly drained and to comply with Policy 38 of the Horsham District Planning Framework (2015).

3 - Regulatory Condition: Within three months of the date of this decision a detailed surface water drainage scheme, which shall include a Surface Water Drainage Statement based on sustainable drainage principles and an assessment of the hydrological and hydrogeological context of the site and development, shall be submitted to the Local Planning Authority for its approval in writing. The submitted details shall be fully coordinated with the landscape. The scheme shall be implemented within 3 months of such approval being given in strict accordance with the agreed details. The drainage scheme shall be retained in that manner thereafter.
Reason: As this matter is fundamental to prevent the increased risk of flooding, to improve and protect water quality, improve habitat and amenity, and ensure future maintenance in accordance Policies 35 and 38 of the Horsham District Planning Framework (2015).

SURFACE WATER

INFILTRATION TESTING WAS UNDERTAKEN AT THE SITE (AS PER DRAINAGE STRATEGY REPORT FROM MR JOE MALONEY DATED FEBRUARY 2026). TESTS UNDERTAKEN SHOW THE SLOWEST RATE TO BE 1.32x10⁻⁶. HOWEVER, DURING EXCAVATIONS GROUNDWATER WAS POTENTIALLY ENCOUNTERED AT >1m DEPTH, AS SUCH INFILTRATION HAS NOT BEEN UTILISED AT THIS TIME.

AS AN ALTERNATIVE, AND IN LINE WITH THE SUDS HIERARCHY, SURFACE WATER ON SITE IS TO BE TREATED AND ATTENUATED WITHIN LINE PERMEABLE SURFACING PRIOR TO RELEASE AT A LIMITED RATE (AS CLOSE AS FEASIBLE TO GREENFIELD) INTO A DITCH THAT RUNS ALONG THE SOUTHEAST PERIMETER.

CAUSEWAY FLOW CALCULATIONS SHOW THAT A MINIMUM SUB-BASE DEPTH OF 0.397m IS REQUIRED TO ATTENUATE THE 1 IN 100 YEAR +45% CC STORM EVENT.

AN ORIFICE PLATE MANHOLE (24mm DIAMETER) IS PROPOSED TO LIMIT OFFSITE SURFACE WATER DISCHARGE TO 0.7L/S INTO THE DITCH. THIS IS TO MATCH GREENFIELD RUNOFF RATES (QBAR) DURING THE CRITICAL (1 IN 100 YEAR +45% CC) STORM EVENT.

ROOF RUNOFF TO FLOW ONTO PERMEABLE SURFACE AND INTO WIDER PROPOSED DRAINAGE SYSTEM

FURTHER DETAILS OF THE DITCH MAY BE REQUIRED TO UNDERSTAND THE FEASIBILITY OF THE OUTFALL LOCATION VIA A TOPOGRAPHIC SURVEY. ALL RELEVANT PERMISSIONS ARE TO BE ACQUIRED PRIOR TO CONSTRUCTION.

FOUL WATER

NEW PACKAGE TREATMENT PLANT INSTALLED AS PER GEORGE WHITE PLANNING DOCUMENT REF DC/25/0982

TREATED WATER PROPOSED TO DISCHARGE VIA COMBINED OUTFALL FROM SITE INTO DITCH

TANK TO BE MAINTAINED AND EMPTIED AS PER MANUFACTURERS GUIDELINES

NOTES

SURFACE ASSUMED TO BE FLAT FOR THE PURPOSES OF THE HYDRAULIC CALCULATIONS AT THIS TIME - TO BE CONFIRMED VIA TOPOGRAPHIC SURVEY

DRAINAGE DITCH LOCATION AS PER CLIENTS DRAWINGS

LEVELS AND FEASIBILITY TO BE CONFIRMED VIA FURTHER INVESTIGATIONS PRIOR TO CONSTRUCTION

ALL RELEVANT CONSENTS/PERMISSIONS TO BE ACQUIRED PRIOR TO CONSTRUCTION

TO BE READ IN CONJUNCTION WITH MCEC CALCULATIONS DOCUMENT 'MC0754 Network Calcs SW FW', MR JOE MALONEY DOCUMENT 'Drainage Strategy' and GEORGE WHITE PLANNING SEWAGE DOCUMENT Ref: DC/25/0982

TYPE C (NO INFILTRATION) PERMEABLE SURFACE

TOTAL CATCHMENT AREA: 930m²
TANKED PERMEABLE SURFACE PLAN AREA: 700m²
MINIMUM SUB-BASE DEPTH: 400mm
CL: 62.00
IL: 61.52

SURFACE ASSUMED TO BE FLAT AT THIS TIME - TO BE CONFIRMED VIA TOPOGRAPHIC SURVEY

GROUNDWATER POTENTIALLY ENCOUNTERED BELOW 1m DEPTH (SEE DRAINAGE STRATEGY REPORT FROM MR JOE MALONEY DATED FEBRUARY 2026) - AS SUCH, INFILTRATION HAS BEEN DISCOUNTED AS A METHOD OF DISCHARGE AT THIS TIME

INFILTRATION MAY BE FEASIBLE IF GROUNDWATER >1m BELOW INVERT OF PERMEABLE SURFACE BASE

ORIFICE PLATE FLOW CONTROL (24mm) LIMITING FLOW TO 0.7L/S DURING THE 1 IN 100 YEAR +45% CLIMATE CHANGE STORM EVENT

THIS IS TO MATCH GREENFIELD RUNOFF RATES (QBAR) DURING THE CRITICAL (1 IN 100 YEAR +45% CC) STORM EVENT

NEW PACKAGE TREATMENT PLANT
TANK TO BE MAINTAINED/EMPTIED AS PER
MANUFACTURERS GUIDELINES

PERFORATED PIPE TO COLLECTED AND CONVEY
SURFACE WATER WITHIN SUB-BASE

DRAINAGE DITCH AS PER CLIENTS DRAWINGS
LEVELS AND FEASIBILITY TO BE CONFIRMED VIA
FURTHER INVESTIGATIONS PRIOR TO CONSTRUCTION
ALL RELEVANT CONSENTS/PERMISSIONS TO BE
ACQUIRED PRIOR TO CONSTRUCTION

MANHOLE COMBINING TREATED WATER FROM
FOUL PACKAGE TREATMENT PLANT AND SURFACE
WATER FROM PERMEABLE SURFACE SUB-BASE PRIOR
TO ENTERING DITCH

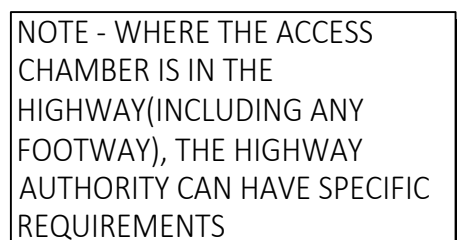
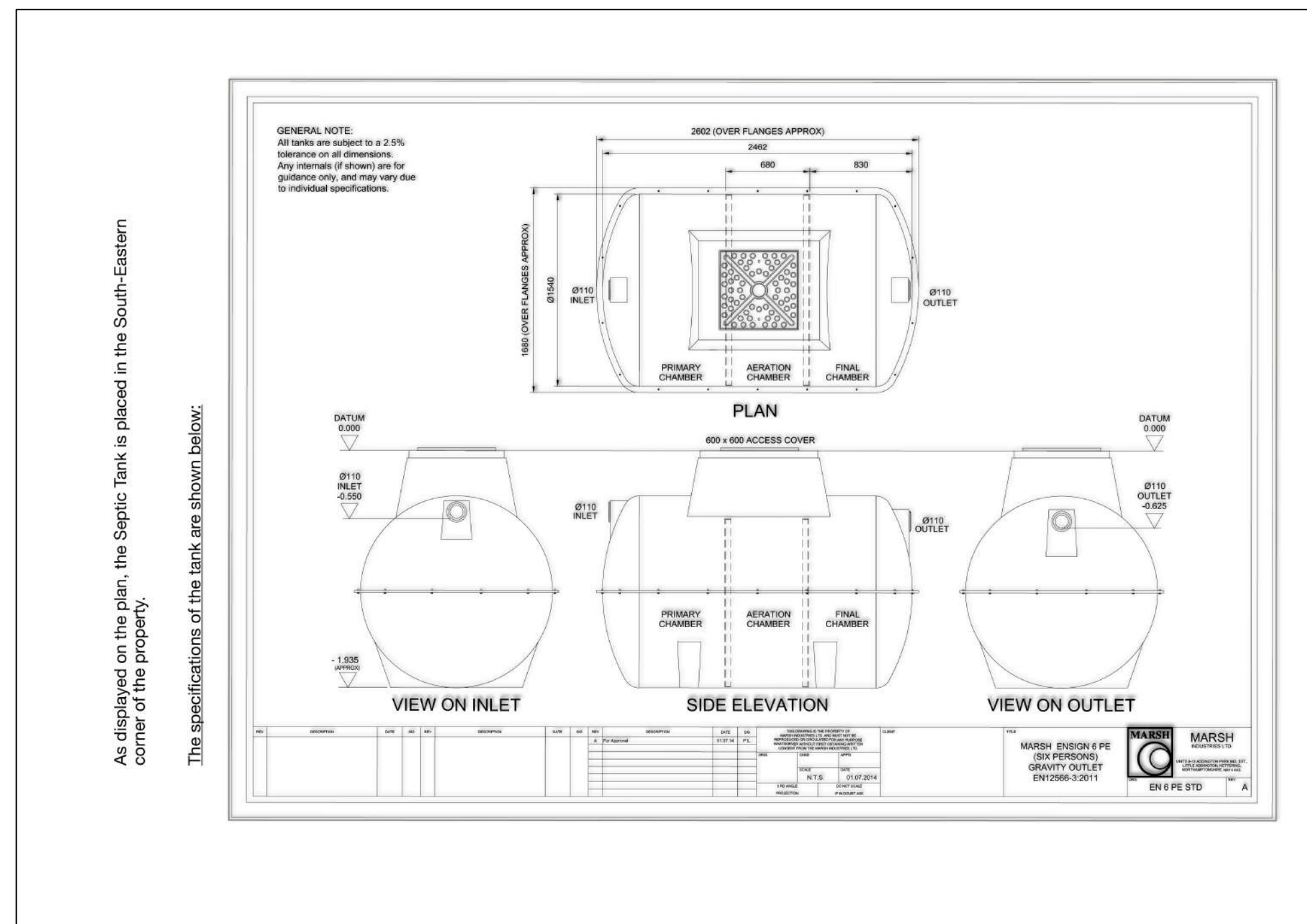
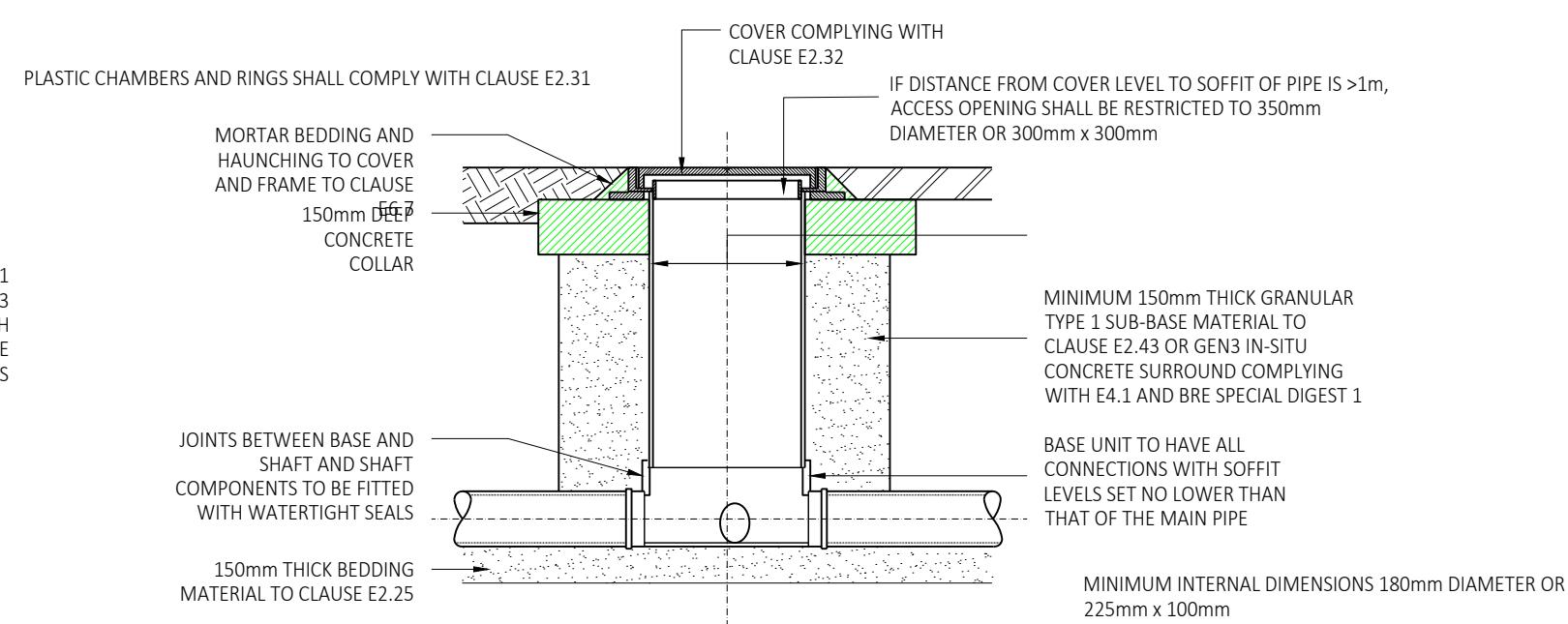
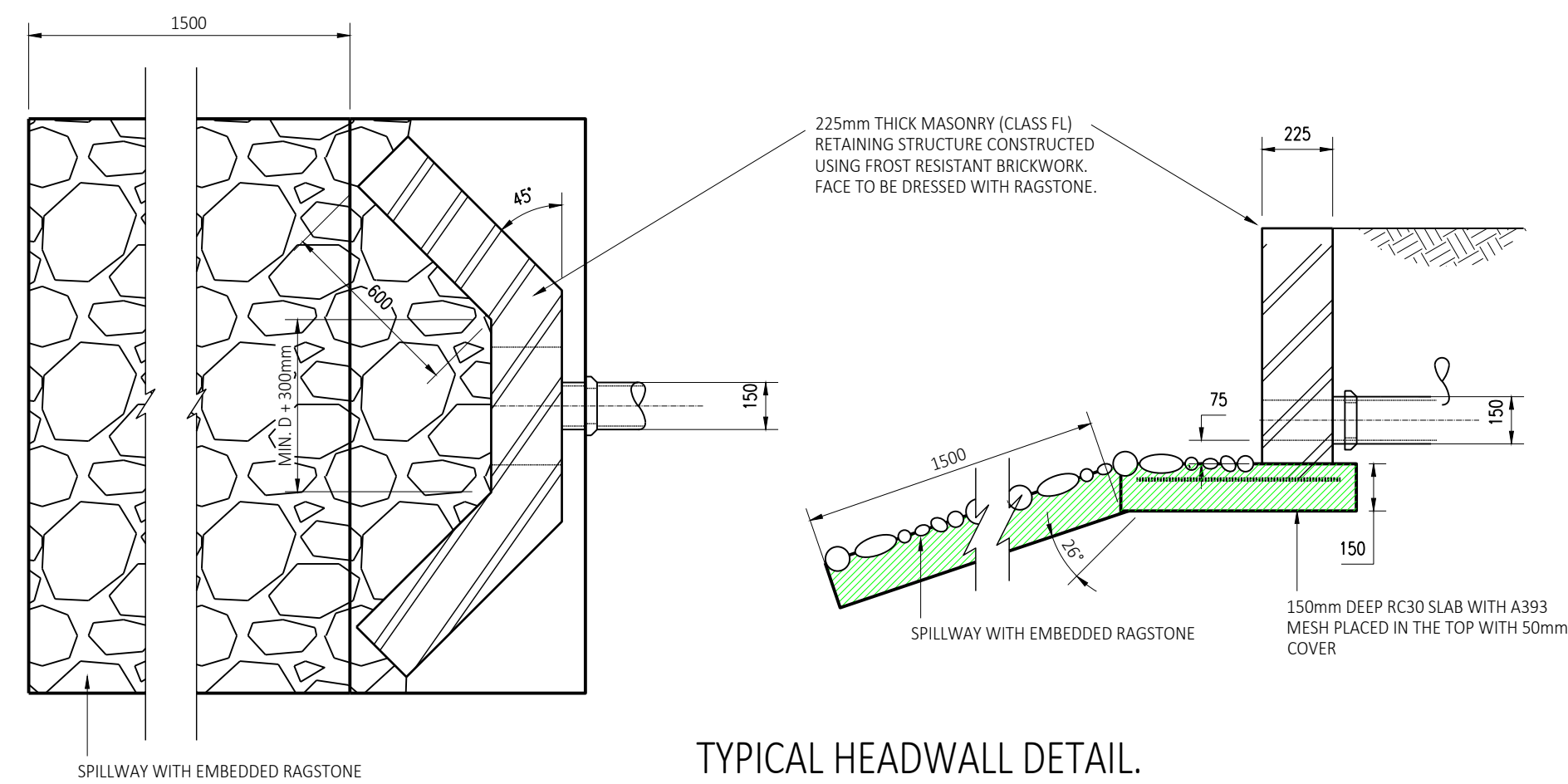
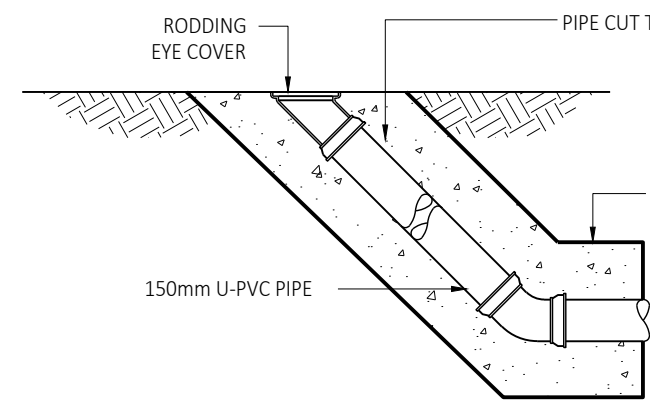
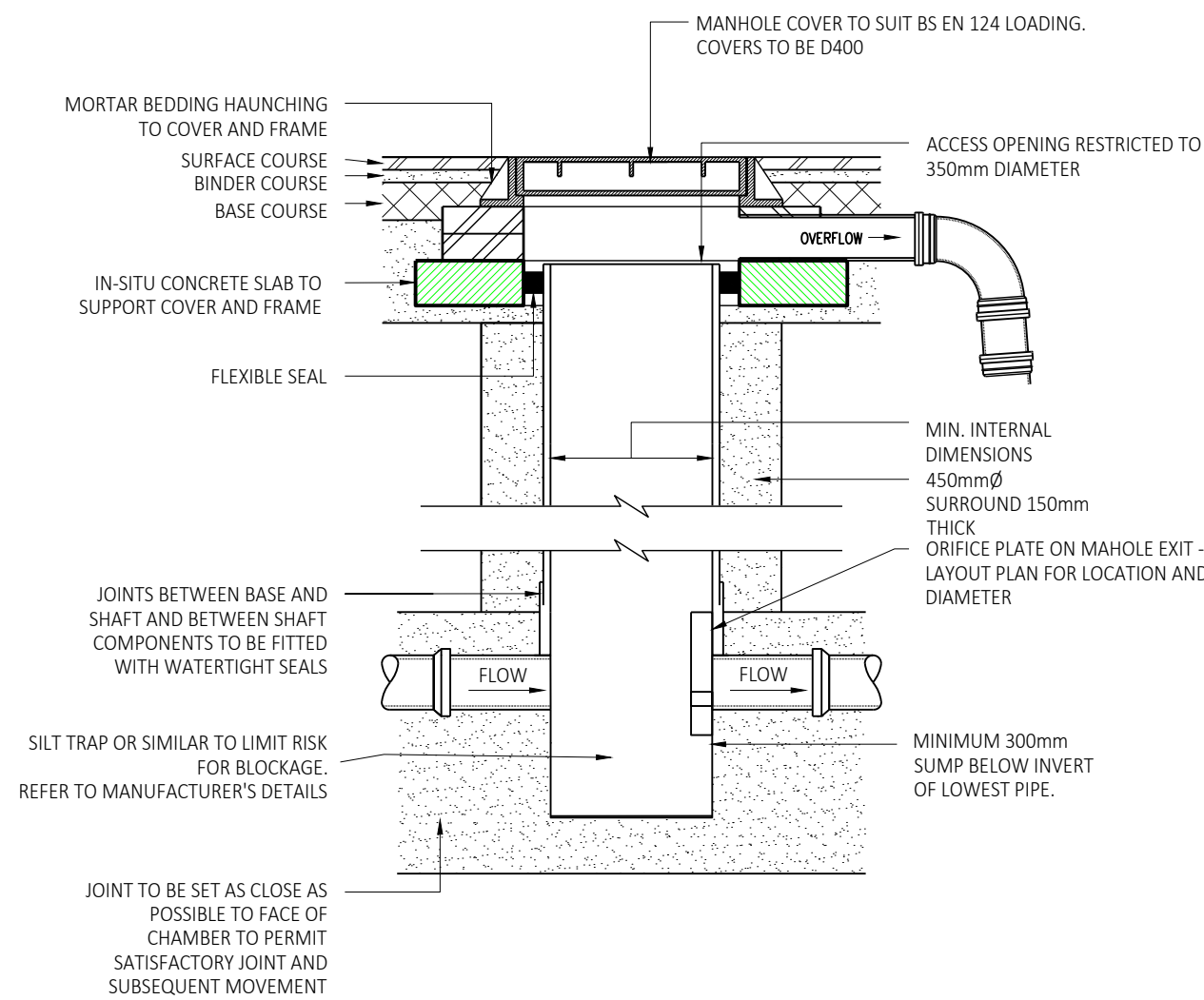
DITCH OUTFALL LOCATION
DITCH FEASIBILITY TO BE CONFIRMED VIA SURVEY
RELEVANT PERMISSIONS TO BE ACQUIRED PRIOR TO
CONSTRUCTION

1. GENERAL
a. THIS DRAWING IS NOT TO BE SCALED, WORK TO FIGURED DIMENSIONS ONLY, CONFIRMED ON SITE.
b. THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL RELEVANT ARCHITECTURAL DRAWINGS, DETAILED SPECIFICATIONS WHERE APPLICABLE AND ALL ASSOCIATED DRAWINGS IN THIS SERIES.
c. ANY DISCREPANCY ON THIS DRAWING IS TO BE REPORTED IMMEDIATELY TO THE PARTNERSHIP FOR CLARIFICATION.
d. THE CONTRACTOR IS RESPONSIBLE FOR ALL TEMPORARY WORKS AND FOR THE STABILITY OF THE WORKS IN PROGRESS.
e. CDM REGULATIONS 2015: ALL CURRENT DRAWINGS AND SPECIFICATIONS MUST BE READ IN CONJUNCTION WITH THE DESIGNER'S HAZARD RISK AND ENVIRONMENT ASSESSMENT RECORD. DESIGN HAS BEEN PRODUCED BASED ON INFORMATION PROVIDED BY THE CLIENT/PRINCIPLE DESIGNER AVAILABLE AT TIME OF ISSUE. CONTRACTOR TO REVIEW DRAWING AND SPECIFICATION IN CONTEXT WITH THE WIDER SITE AND SPECIFIC SITE INVESTIGATION, CONTAMINATION ASSESSMENT, ASBESTOS SURVEY, ENVIRONMENTAL SURVEY, UXO SURVEY AND ANY OTHER RELEVANT INFORMATION AND MANAGE RISKS RELATING TO THE WORKS OUTLINED IN THE DRAWINGS AND SPECIFICATION. PRINCIPLE CONTRACTOR TO MAKE DESIGNER AND CLIENT AWARE OF SITE SPECIFIC RISKS THAT MAY AFFECT THE DRAWING AND SPECIFICATION.
f. CDM REGULATIONS 2015: FOR GENERIC MAINTENANCE AND MANAGEMENT RISKS REFER TO CHAPTER 36 OF CIRIA 752 SUDS MANUAL. FOR PROPRIETARY SYSTEMS SEE MANUFACTURER'S MANAGEMENT AND MAINTENANCE DETAILS AND RISK ASSESSMENT WITH REGARDS TO MAINTENANCE OF PROPRIETARY SYSTEMS.
2. CONSTRUCTION NOTE
a. THE MAIN CONTRACTOR IS RESPONSIBLE FOR THE DESIGN OF ALL TEMPORARY WORKS, AND IS ALSO RESPONSIBLE FOR THE SAFE MAINTENANCE AND STABILITY OF EXISTING BUILDINGS AT ALL TIMES.
b. THE MAIN CONTRACTOR IS RESPONSIBLE FOR ALL OCCURRENCES OF GROUND WATER DURING THE CONSTRUCTION PERIOD.
c. ANY INFORMATION GIVEN REGARDING EXISTING UNDERGROUND SERVICES IS GIVEN IN GOOD FAITH AFTER CONSULTATION WITH THE RELEVANT AUTHORITY, HOWEVER ACCURACY IS NOT CERTAIN. THE MAIN CONTRACTOR IS RESPONSIBLE FOR CHECKING ALL INFORMATION ON SITE PRIOR TO WORK COMMENCING AND TAKING DUE CARE AND ATTENTION WHILST UNDERTAKING THE WORKS.
d. THE CONTRACTOR MUST COMPLY WITH ALL CURRENT LEGISLATION RELATING TO HEALTH & SAFETY.
e. ALL PRODUCTS SPECIFIED SHALL BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND INSTRUCTIONS. IF THERE ARE DISCREPANCIES BETWEEN THAT INFORMATION AND THE DETAILS ON ANY MERIDIAN DRAWINGS, THE MANUFACTURER'S INSTRUCTIONS MUST BE USED.
3. BELOW GROUND DRAINAGE
a. UPVC-U PIPES TO BS 45660 : 2000 AND PLASTIC INSPECTION CHAMBERS AND FITTINGS TO BS EN 13598-1:2020. CLAY PIPES TO BS EN 295-1:2013. CONCRETE MANHOLE AND INSPECTION CHAMBERS TO BS EN 1917:2002
b. ALL ADAPTABLE DRAINAGE TO BE CONSTRUCTED IN ACCORDANCE WITH SEWERAGE SECTOR GUIDANCE App C - DESIGN AND CONSTRUCTION GUIDANCE AND THE RELEVANT COUNCIL DESIGN GUIDE.
c. ALL PRIVATE FOUL WATER SEWERS TO BE LAID AT 1 IN 40 AT THE HEAD OF PIPE RUNS AND 1 IN 80 ELSEWHERE UNLESS OTHERWISE STATED.
d. ALL PRIVATE FOUL SEWER PIPES TO BE 100mm DIAMETER FROM SOIL STACKS UNLESS OTHERWISE STATED ON THE DRAWING AND 150mm WHERE SERVING MORE THAN 9 PROPERTIES.
e. ALL PRIVATE SURFACE WATER SEWERS TO BE LAID AT 1 IN 100 UNLESS OTHERWISE STATED ON THE DRAWING.
f. ALL PRIVATE SURFACE WATER SEWER PIPES TO BE 100mm DIAMETER FROM DOWNPIPES AND 150mm DIAMETER ELSEWHERE UNLESS OTHERWISE STATED ON THE DRAWING.
g. ALLOW FOR RODDING ACCESS ABOVE GROUND WHERE RAINWATER DOWNPIPES OR SOIL STACKS DO NOT HAVE A DIRECT CONNECTION TO AN INSPECTION CHAMBER.
h. EXISTING SEWER PIPE TO BE RE-USED TO BE SURVEYED AND LEVELED PRIOR TO COMMENCEMENT OF THE DRAINAGE WORKS AND REFURBISHED IF NECESSARY.
i. CONNECTIONS TO AN ADOPTED SEWER ONLY TO BE MADE FOLLOWING APPROVAL FROM THE RELEVANT ADOPTING AUTHORITY.
j. ALL DRAINS, SEWER PIPES AND MANHOLES TO BE CLEANED AND TESTED FOR WATER TIGHTNESS ON COMPLETION OF CONSTRUCTION.
4. MANHOLE COVERS AND FRAMES
a. MANHOLE COVERS TO BE CLASS D400 IN HIGHWAYS, CLASS B125 IN FOOTWAYS AND VERGES, CLASS A15 IN NON-TRAFFICKED AREAS.
b. MANHOLE COVER AND FRAME TO BE BEDDED AND SURROUNDED IN 1:3 MORTAR.

LEGEND

- EXISTING DITCH AS PER CLIENTS DRAWING PROVIDED
- PROPOSED FW PIPE RUN
- PROPOSED SW PIPE RUN
- COMBINED RUN
- PROPOSED PERFORATED SW DRAIN
- PROPOSED ORIFICE PLATE MANHOLE
- PACKAGED TREATMENT PLANT
- HEADWALL
- PERMEABLE PAVING - TANKED
- PROPOSED TYPE D FW INSPECTION CHAMBER
- PROPOSED TYPE E FW INSPECTION CHAMBER
- COMBINED MANHOLE

-	-	-	-
REV:	DESCRIPTION:	BY:	DATE:
STATUS: Preliminary			
MERIDIAN CIVIL ENGINEERING CONSULTANCY			
CLIENT: Mr Maloney			
SITE: Land East of Marches Road, Horsham, West Sussex, RH12 3SH			
TITLE: Surface and Foul Water Drainage Strategy - Proposed Layout			
SCALE AT A1: 1:100	DATE: 12/02/2026	DRAWN: SL	CHECKED: BB
PROJECT NO: MC0754	DRAWING NO: CIV01	REVISION: -	



1. **GENERAL**

a. THIS DRAWING IS NOT TO BE SCALED, WORK TO FIGURED DIMENSIONS ONLY, CONFIRMED ON SITE.

b. THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL RELEVANT ADDITIONAL DRAWINGS, DETAIL SPECIFICATIONS WHERE APPLICABLE AND ALL ASSOCIATED DRAWINGS IN THIS SERIES.

c. ANY DISCREPANCY ON THIS DRAWING IS TO BE REPORTED IMMEDIATELY TO THE PARTNERSHIP FOR CLARIFICATION.

d. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL TEMPORARY WORKS AND FOR THE STABILITY OF THE WORKS IN PROGRESS.

e. CDM REGULATIONS 2015. ALL CURRENT DRAWINGS AND SPECIFICATIONS MUST BE READ IN CONJUNCTION WITH THE DESIGNER'S HAZARD RISK AND ENVIRONMENTAL ASSESSMENT RECORD. DESIGN HAS BEEN PROVIDED BASED ON INFORMATION PROVIDED BY THE CLIENT/PRINCIPLE DESIGNER AVAILABLE AT TIME OF ISSUE. CONTRACTOR TO REVIEW DRAWING AND SPECIFICATION IN CONTEXT WITH THE WIDER SITE AND SPECIFIC SITE CONDITIONS, COMMUNITY, ENVIRONMENTAL, ASBESTOS SURVEY, ENVIRONMENTAL SURVEY, GEO SURVEY, AND ANY OTHER RELEVANT INFORMATION AND MANAGE RISKS RELATING TO THE WORKS OUTLINED IN THE DRAWINGS AND SPECIFICATION. PRINCIPLE CONTRACTOR TO MAKE DISCREPANCY CLIENT AWARE OF SITE SPECIFIC RISKS THAT MAY AFFECT THE DRAWING AND SPECIFICATION.

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2. **CONSTRUCTION NOTE**

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b. THE MAIN CONTRACTOR IS RESPONSIBLE FOR ALL OCCURRENCES OF GROUND COLLAPSE DURING THE CONSTRUCTION OF THE WORKS.

c. ANY INFORMATION GIVEN REGARDING EXISTING UNDERGROUND SERVICES IS GIVEN IN GOOD FAITH AFTER CONSULTATION WITH THE RELEVANT AUTHORITY, HOWEVER ACCURACY IS NOT CERTAIN. THE MAIN CONTRACTOR IS RESPONSIBLE FOR CHECKING ALL INFORMATION ON SITE PRIOR TO WORK COMMENCING AND TAKING DUE CARE AND ATTENTION WHILE UNDERTAKING THE WORKS.

d. THE CONTRACTOR MUST COMPLY WITH ALL CURRENT LEGISLATION RELATING TO HEALTH AND SAFETY.

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3. **BELOW GROUND DRAINAGE**

a. UPRC PIPES TO BE 400mm 2000 AND PLASTIC INSPECTION CHAMBERS AND FITTINGS TO BS EN 13598-12000. CLAY PIPES TO BS EN 1937-2013. CONCRETE MANHOLE AND INSPECTION CHAMBERS TO BS EN 1937-2002.

b. ALL ADAPTABLE DRAINAGE TO BE CONSTRUCTED IN ACCORDANCE WITH SEWERAGE SCHEME GUIDANCE APP C - DESIGN AND CONSTRUCTION AND UPRC AND THE RELEVANT MANUFACTURERS INSTRUCTIONS.

c. ALL PRIVATE FOUL WATER SEWERS TO BE LAID AT 1 IN 40 AT THE HEAD OF PIPE RUNS AND 1 IN 80 ELSEWHERE UNLESS OTHERWISE STATED.

d. ALL PRIVATE FOUL SEWER PIPES TO BE 100mm DIAMETER FROM SOIL STACKS DOWNWARDS TO 150mm DIAMETER ELSEWHERE. THE DRAINING AND 150mm WHERE SERVING MORE THAN 9 PROPERTIES.

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4. **MANHOLE COVERS AND FRAMES**

a. MANHOLE COVERS TO BE CLASS D400 IN HIGHWAYS, CLASS B125 IN FOOTWAYS AND VERGES, CLASS A15 IN NON-TRAFFICKED AREAS.

b. MANHOLE COVER AND FRAME TO BE BEDDED AND SURROUNDED IN 1:3 MORTAR.

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REV:	DESCRIPTION:			DATE:
STATUS:				
Preliminary				
MERIDIAN				
CIVIL ENGINEERING CONSULTANCY				
CLIENT: Mr Maloney				
SITE:	Land East of Marches Road, Horsham, West Sussex, RH12 3SH			
TITLE:	Surface and Foul Water Drainage Strategy - Typical Details			
SCALE AT A1:	DATE:	DRAWN:	CHECKED:	
1:20	12/02/2024	SL	BB	
PROJECT NO:	DRAWING NO:		REVISION:	
MC0754	CIV02		-	

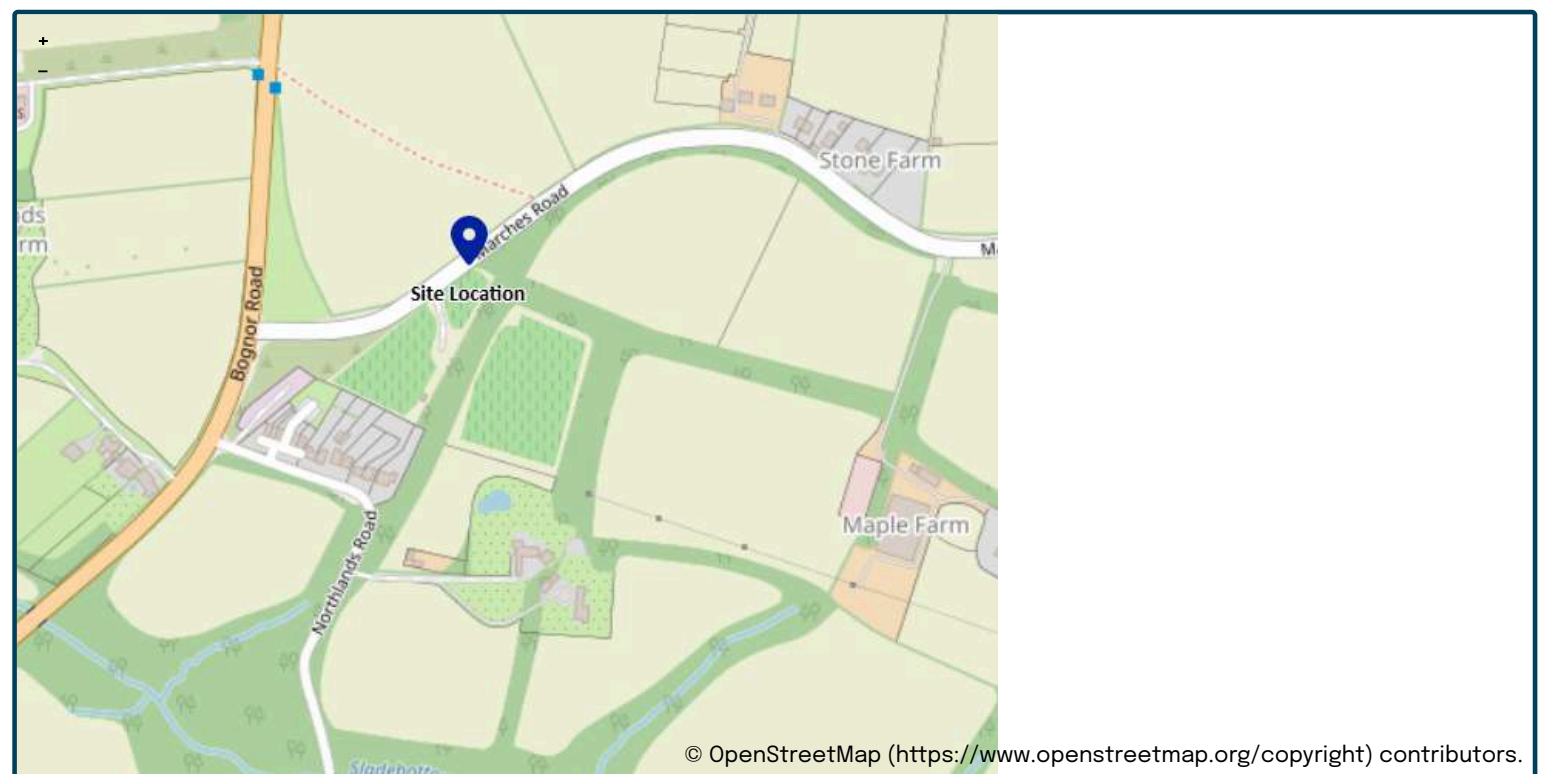
This is an estimation of the greenfield runoff rates that are used to meet normal best practice criteria in line with Environment Agency guidance “Rainfall runoff management for developments”, SC030219 (2013), the SuDS Manual C753 (CIRIA, 2015) and the non-statutory standards for SuDS (Defra, 2015). This information on greenfield runoff rates may be the basis for setting consents for the drainage of surface water runoff from sites.

Project details

Date	16/02/2026
Calculated by	SL
Reference	Marches Rd
Model version	2.2.2

Location

Site name	Marches Rd
Site location	Horsham



Site easting (British National Grid)	514734
Site northing (British National Grid)	135630

Site details

Total site area (ha)	0.0712	ha
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MORE INFO

Greenfield runoff

Method

Method	FEH statistical (2025)
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FEH statistical (2025)

	My value	Map value
SAAR9120 (mm)	844	mm
BFIHOST19scaled	0.236	
QMed-QBar conversion	1.136	1.136
QMed (l/s)	0.6	l/s
QBar (FEH statistical 2025) (l/s)	0.7	l/s

Growth curve factors

	My value	Map value
Hydrological region	7	7
1 year growth factor	0.85	
2 year growth factor	0.88	
10 year growth factor	1.62	
30 year growth factor	2.3	
100 year growth factor	3.19	
200 year growth factor	3.74	

Results

Method	FEH statistical (2025)
Flow rate 1 year (l/s)	0.6
Flow rate 2 year (l/s)	0.6
Flow rate 10 years (l/s)	1.1
Flow rate 30 years (l/s)	1.5
Flow rate 100 years (l/s)	2.1
Flow rate 200 years (l/s)	2.4

Please note runoff estimation is subject to significant uncertainty. Results are therefore normally reported to only 1 decimal place. Where 2 decimal places are provided, this does not indicate accuracy to this level, it has been adopted to prevent ‘zero’ figures from being reported. Outputs less than 0.01 l/s are reported as 0.01 l/s.

Disclaimer

This report was produced using the Greenfield runoff rate estimation tool (2.2.2) developed by HR Wallingford and available at uksuds.com (<https://www.uksuds.com/>). The use of this tool is subject to the UK SuDS terms and conditions and licence agreement, which can both be found at [uksuds.com/terms-conditions](https://www.uksuds.com/terms-conditions) (<https://www.uksuds.com/terms-conditions>). The outputs from this tool have been used to estimate Greenfield runoff rates. The use of these results is the responsibility of the users of this tool. No liability will be accepted by HR Wallingford, the Environment Agency, Centre for Ecology and Hydrology, Wallingford Hydrosolutions or any other organisation for the use of these data in the design or operational characteristics of any drainage scheme.

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Design Settings

Rainfall Methodology	FEH-22	Minimum Velocity (m/s)	1.00
Return Period (years)	100	Connection Type	Level Soffits
Additional Flow (%)	0	Minimum Backdrop Height (m)	0.200
CV	1.000	Preferred Cover Depth (m)	1.200
Time of Entry (mins)	4.00	Include Intermediate Ground	✓
Maximum Time of Concentration (mins)	30.00	Enforce best practice design rules	x
Maximum Rainfall (mm/hr)	50.0		

Simulation Settings

Rainfall Methodology	FEH-22	Analysis Speed	Normal	Starting Level (m)	
Rainfall Events	Singular	Skip Steady State	x	Check Discharge Rate(s)	x
Summer CV	1.000	Drain Down Time (mins)	1440	Check Discharge Volume	x
Winter CV	1.000	Additional Storage (m³/ha)	0.0		

Storm Durations

15 | 30 | 60 | 120 | 180 | 240 | 360 | 600 | 960 | 1440 | 2160

Return Period (years)	Climate Change (CC %)	Additional Area (A %)	Additional Flow (Q %)
2	0	0	0
30	0	0	0
100	0	0	0
100	45	0	0

Node Orifice Plate MH Online Orifice Control

Flap Valve	x	Invert Level (m)	61.274	Diameter (m)	0.024
Downstream Link	1.001	Design Depth (m)	0.400	Discharge Coefficient	0.600
Replaces Downstream Link	x	Design Flow (l/s)	0.8		

Results for 2 year Critical Storm Duration. Lowest mass balance: 98.93%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	RE1	1	61.520	0.000	0.0	0.0000	0.0000	OK
15 minute summer	RE2	1	61.570	0.000	0.0	0.0000	0.0000	OK
1440 minute summer	Orifice Plate MH	960	61.375	0.101	1.5	21.1268	0.0000	OK
1440 minute summer	Combined MH	990	60.815	0.015	0.4	0.0023	0.0000	OK
1440 minute summer	5	990	60.765	0.015	0.4	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
15 minute summer	RE1	1.000	Orifice Plate MH	0.0	0.000	0.000	0.0548	
15 minute summer	RE2	2.000	Orifice Plate MH	0.0	0.000	0.000	0.0000	
1440 minute summer	Orifice Plate MH	1.001	Combined MH	0.4	0.345	0.024	0.0113	
1440 minute summer	Combined MH	1.002	5	0.4	0.398	0.020	0.0043	32.2

Results for 30 year Critical Storm Duration. Lowest mass balance: 99.27%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	RE1	1	61.520	0.000	0.0	0.0000	0.0000	OK
15 minute summer	RE2	1	61.570	0.000	0.0	0.0000	0.0000	OK
600 minute winter	Orifice Plate MH	570	61.481	0.207	3.8	43.2696	0.0000	SURCHARGED
600 minute winter	Combined MH	570	60.818	0.018	0.5	0.0028	0.0000	OK
600 minute winter	5	570	60.768	0.018	0.5	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
15 minute summer	RE1	1.000	Orifice Plate MH	0.0	0.000	0.000	0.1955	
15 minute summer	RE2	2.000	Orifice Plate MH	0.0	0.000	0.000	0.0000	
600 minute winter	Orifice Plate MH	1.001	Combined MH	0.5	0.388	0.036	0.0148	
600 minute winter	Combined MH	1.002	5	0.5	0.448	0.029	0.0057	43.8

Results for 100 year Critical Storm Duration. Lowest mass balance: 99.23%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
960 minute winter	RE1	885	61.538	0.018	0.0	0.0003	0.0000	OK
15 minute summer	RE2	1	61.570	0.000	0.0	0.0000	0.0000	OK
960 minute winter	Orifice Plate MH	885	61.538	0.264	3.3	55.2675	0.0000	SURCHARGED
960 minute winter	Combined MH	885	60.819	0.019	0.6	0.0030	0.0000	OK
960 minute winter	5	885	60.769	0.019	0.6	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
960 minute winter	RE1	1.000	Orifice Plate MH	0.0	-0.001	-0.001	0.3458	
15 minute summer	RE2	2.000	Orifice Plate MH	0.0	0.000	0.000	0.0000	
960 minute winter	Orifice Plate MH	1.001	Combined MH	0.6	0.403	0.041	0.0163	
960 minute winter	Combined MH	1.002	5	0.6	0.466	0.033	0.0062	59.8

Results for 100 year +45% CC Critical Storm Duration. Lowest mass balance: 99.42%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
960 minute winter	RE1	900	61.671	0.151	0.0	0.0027	0.0000	SURCHARGED
960 minute winter	RE2	900	61.671	0.101	0.0	0.0018	0.0000	OK
960 minute winter	Orifice Plate MH	915	61.671	0.397	4.8	83.2772	0.0000	SURCHARGED
960 minute winter	Combined MH	915	60.821	0.021	0.7	0.0033	0.0000	OK
960 minute winter	5	915	60.771	0.021	0.7	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
960 minute winter	RE1	1.000	Orifice Plate MH	0.0	-0.004	-0.003	0.6488	
960 minute winter	RE2	2.000	Orifice Plate MH	0.0	0.005	0.003	0.1445	
960 minute winter	Orifice Plate MH	1.001	Combined MH	0.7	0.430	0.051	0.0189	
960 minute winter	Combined MH	1.002	5	0.7	0.496	0.041	0.0072	77.4

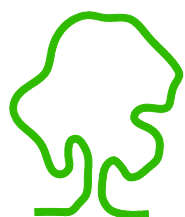
MR JOE MALONEY



SITE AT MARCHES ROAD, WARNHAM, WEST SUSSEX, RH12 3SL

Drainage Strategy

February 2026



eas ltd

Environmental Assessment Services Ltd

REPORT DATA SHEET

Requirement	Data
Report Reference	875/WhitePlanning/MarchesRoad/DS
Date	February 2026
Client	Mr Joe Maloney
Report type	Drainage Strategy
Purpose	Planning
Revisions	
Prepared by	Eur Ing Malcolm McKemey BSc (Hons), CEng, CEnv, MICE, MCIWEM, MIEEnvSc  Signed
Approved by	Xanthe Lyford BSc (Hons)  Signed

MR JOE MALONEY

SITE AT MARCHES ROAD, WARNHAM, WEST SUSSEX, RH12 3SL

Drainage Strategy

February 2026

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5. FOUL SEWERAGE
6. MAINTENANCE
7. CONCLUSIONS
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MR JOE MALONEY

SITE AT MARCHES ROAD, WARNHAM, WEST SUSSEX, RH12 3SL

Drainage Strategy

February 2026

1. EXISTING SITE & PROPOSED DEVELOPMENT

- 1.1 The site comprises a plot of land on the east site of Marches Road near the junction with the A29, some 2 km north of the Village of Warnham. The site area is approximately 0.15 ha. The Ordnance Survey (OS) map reference for the site is TQ 1470 3555. The site elevation is approximately +60 m OD. See Appendix A, Figure 1: Site Location Plan and Figure 2: Site Layout & Testing Locations.
- 1.2 It is proposed to locate residential static caravans/park homes and associated infrastructure on the site.
- 1.3 According to the British Geological Survey (available online), the site lies on Cretaceous Weald Clay (mudstone). The low permeability Weald Clay is classified as unproductive strata. The site does not lie within a groundwater source protection zone.
- 1.4 There is a degraded ditch on the other side of Marches Road, but otherwise no nearby watercourses are apparent.

2. FLOOD RISK ASSESSMENT

- 2.1 According to the EA online Flood Map for Planning, the site lies within Flood Zone 1 (low risk of flooding from fluvial sources).
- 2.2 The EA online Risk of Flooding from Surface Water map shows that the eastern part of the site is at low risk of surface water (pluvial) flooding. The remainder of the site is at very low risk of pluvial flooding
- 2.3 As the site is less than 1 ha in area and lies within Flood Zone 1, there is no requirement for a site specific flood risk assessment under the National Planning Policy Framework (NPPF) and associated guidance documents.
- 2.4 In this case, it is required by Horsham District Council that all runoff for the 1 in 100 year flood event, plus allowance for climate change, will be managed on site, and that that discharge from the proposed development will be restricted to Greenfield run-off rate.

3. EXISTING DRAINAGE

- 3.1 The site was visited on Friday 23 January 2026 in order to examine the existing drainage and carry out percolation testing. See Appendix B: Site Photographs.
- 3.2 Formal surface water drainage on the site is limited to an area paved in an informal drainage blanket of gravel over road planings on a permeable geotextile. It was noted that this arrangement had drained very well despite a long period of heavy rain preceding the site visit.
- 3.3 As a general rule; the flow permitted to leave the developed site should not exceed the Greenfield flow rate. The foul sewage should also be allowed for in the permitted discharge flow rate.
- 3.4 The Greenfield runoff rate for the site has therefore been estimated using the UK Sustainable Drainage (UKSUDS) Greenfield Runoff Estimation Tool. The runoff rates are given for scenarios up to (and including) the 1 in 200 year storm event, as summarised in Table 3.1 below and provided in Appendix D.

TABLE 3.1
GREENFIELD RUNOFF RATES

Return period	Flow rates (l/s)
1 in 1 year	0.7
1 in 2 year	0.7
1 in 10 year	1.3
1 in 30 year	1.9
1 in 100 year	2.7
1 in 200 year	3.1

- 3.5 The DEFRA Non-statutory technical standards for sustainable drainage systems clause S2 recommends that the discharge from the site should not exceed the 1 in 1 year return period flow (0.7 l/s).

4. PROPOSED DRAINAGE

- 4.1 Any surface water drainage scheme will have to be sustainable (SuDS). The hierarchy of SuDS measures is as follows (most desirable first):
- Re-use of rainwater.
 - Discharge of rainwater to infiltration (drainage to soakaway).
 - Attenuation storage with outflow limited to greenfield flow, discharged to a watercourse.
 - Attenuation storage with outflow limited to greenfield flow, discharged to a surface water sewer.
 - Attenuation storage with outflow limited to greenfield flow, discharged to a combined sewer

- 4.2 Apart from collecting roof water for possible garden irrigation or washdown purposes, option (i) does not appear to be practicable.
- 4.3 Drainage to soakaway (ii) requires the underlying soil to be permeable. Excavation in the site revealed a shallow layer of made ground over silty clay.
- 4.4 Three test pits were excavated at locations potentially available for soakaways within the site and percolation testing was carried out. The infiltration coefficients calculated from the percolation testing are summarised in Table 4.1 below:

TABLE 4.1
INFILTRATION COEFFICIENTS

Test Pit No.	1	2	3
<i>f</i> m/s	1.32×10^{-6}	2.19×10^{-6}	2.19×10^{-6}

- 4.5 The results are probably too low for a conventional soakaway solution and water seeping into the excavations below 1 m depth may indicate a groundwater level that is rather high for this option. However, a hybrid solution may be the best option for this site (see below).

4.6 Attenuation Storage

- 4.6.1 The alternative to soakaway drainage would be attenuation storage onsite, with discharge limited to the Greenfield flow for the impermeable areas (in this case 0.7 l/s). This would require a discharge point, an outlet to a watercourse or sewer. The only potential outlet for attenuation storage would be the degraded ditch on the opposite side of Marches Road. It would be necessary to reform the degraded ditch (which is not within the same ownership) plus lay a pipe across the highway.

- 4.6.2 Where discharge to the drainage ditch is required, it will be necessary to obtain an ordinary watercourse land discharge consent from Horsham District Council.

4.7 Infiltration Blanket

- 4.7.1 In this case, providing a permeable surface to the whole site appears the preferred option, and has been found to work in practice, where the existing gravel on road planings over geotextile has retained a dry surface despite a period of continuous rainfall and where adjacent areas were waterlogged.
- 4.7.2 The permeable surface should be formed as a drainage (infiltration) blanket. The blanket medium should be a low-fines gravel, stone or no-fines crushed recycled concrete to a minimum depth of 250 mm laid over a permeable geotextile (Terram 1000 or similar). This is the recommended drainage strategy for this site.

4.8 Other Sustainable Drainage System (SuDS) Options

- 4.8.1 Rainwater harvesting could also be practiced. Rainwater could be used for irrigation purposes within any garden areas, via water butts connected to roof downpipes.
- 4.8.2 It may be possible to incorporate a green roof into the proposed development. Green roofs reduce the volume and rate of runoff and can be low maintenance, generally requiring bi-annual maintenance visits.

4.9 Extreme event

- 4.9.1 If an extreme storm event overwhelmed the surface water drainage system at the site and/or if the drainage system were to fail, ground levels would indicate that surface water would discharge to the north and west, following the slope of the land towards the A29 highway.

5 FOUL SEWERAGE

- 5.1 There is existing foul sewerage at the site, comprising a new package sewage treatment plant located in the northwest corner of the site.

6 MAINTENANCE

- 6.1 The infiltration blanket should require minimal maintenance, beyond avoiding any build-up of surface deposits which could reduce the permeability of the blanket.
- 6.2 The infiltration blanket will provide a degree of pollution control as it will partially act as a filter bed, allowing the breakdown of organic deposits within the blanket matrix.
- 6.3 In the longer run, deterioration of the infiltration blanket may occur due to deposits blocking the gaps between the stones. Should this occur (indicated by a loss in performance), the stone may be lifted and put through a power screen to remove fine material.
- 6.4 Maintenance will be the responsibility of the site owner.

7. CONCLUSIONS

- 7.1 The site comprises a plot of land on the east side of Marches Road near the junction with the A29, some 2 km north of the Village of Warnham. The site area is approximately 0.15 ha. It is proposed to locate residential static caravans/park homes and associated infrastructure on the site.

- 7.2 The site lies within Flood Zone 1, at low risk of fluvial flooding, and the EA online Risk of Flooding from Surface Water map shows that the eastern part of the site is at low risk of surface water (pluvial) flooding.
- 7.3 There is presently no formal surface water drainage on site. Surface water from the site either soaks into the ground or flows to the northwest.
- 7.4 Percolation testing was carried out at three locations across the site. The infiltration coefficients recorded were marginal for a conventional soakaway and seepage into the test holes at around 1 m below ground level suggested a fairly high effective groundwater level.
- 7.5 The nearest watercourse is a degraded ditch beneath the hedgerow on the north side of Marches Road. There are no public sewers in the vicinity. The option of attenuation storage requires controlled discharge to a watercourse or sewer, and it would be necessary to excavate the ditch on the other side of Marches Road to provide a suitable outlet.
- 7.6 The preferred sustainable (SUDS) surface water drainage solution for the site will be providing a permeable surface across the site in the form of an infiltration blanket. This option has been partially tried out at the site, and found to work well following a period of heavy rain. Using the MasterDrain model, the depth of the infiltration blanket should not be less than 250 mm.
- 7.7 Additional SUDS options which could be incorporated into the development include rainwater harvesting for garden irrigation purposes, via the provision of water butts, and a green roof on a dwelling.
- 7.8 There is foul sewerage within the site, comprising a new package sewage treatment plant.



APPENDIX A: FIGURES

- Figure 1: Site Location Plan**
- Figure 2: Site Layout & Testing Locations**
- Figure 3: Proposed Drainage Arrangement**
- Figure 4: Infiltration Blanket Details**



Reproduced from the 1:25000 Scale OS Map with permission. Licence No. 100005508



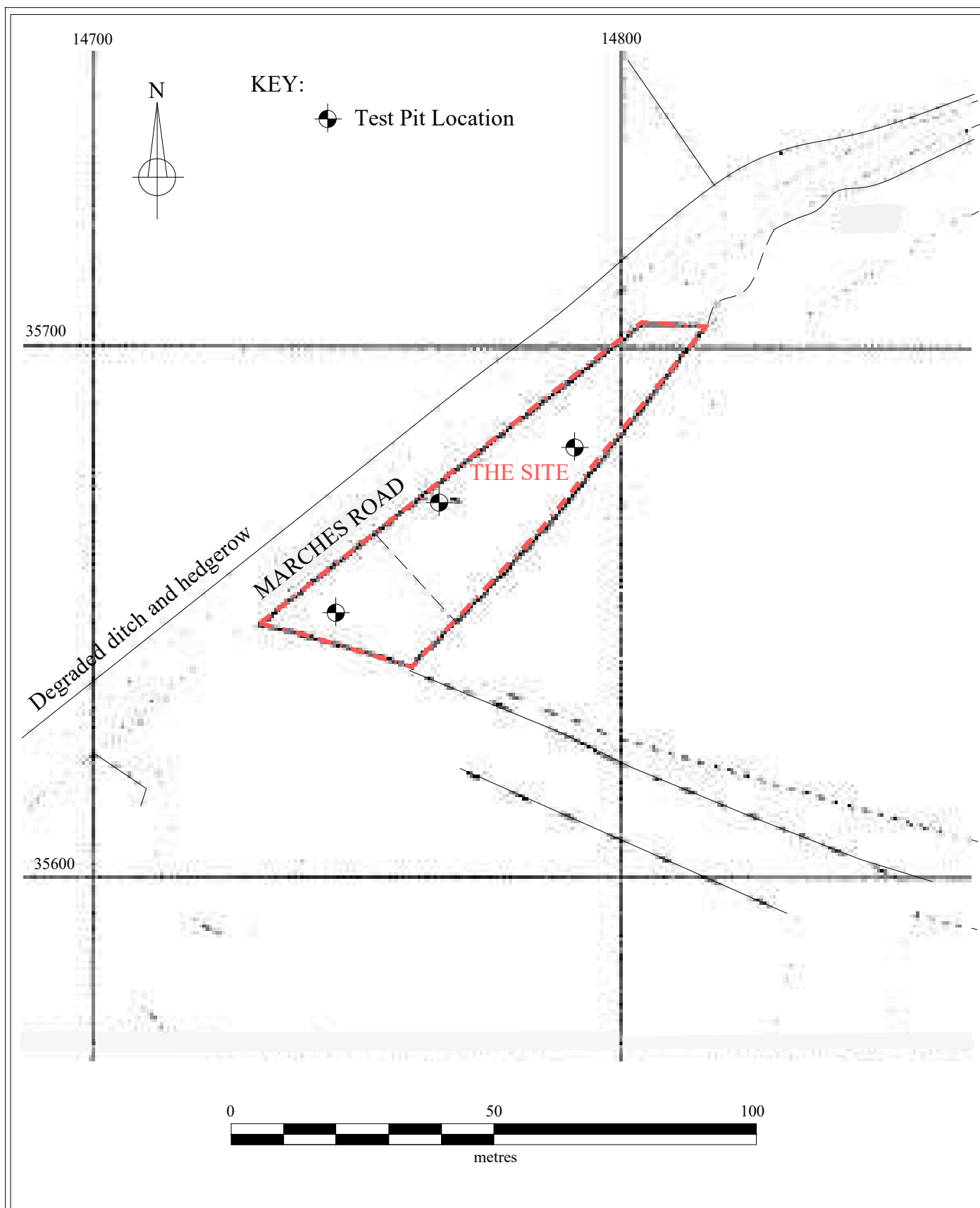
MR JOE MALONEY

Scale 1:25000

SITE AT MARCHES ROAD, WARNHAM, WEST SUSSEX, RH12 3SE

Figure 1: Site Location

February 2026



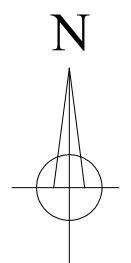
MR JOE MALONEY

Scale as shown

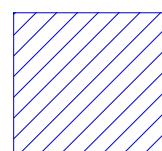
SITE AT MARCHES ROAD, WARNHAM, WEST SUSSEX, RH12 3SE

Figure 2: Site Layout & Testing Locations

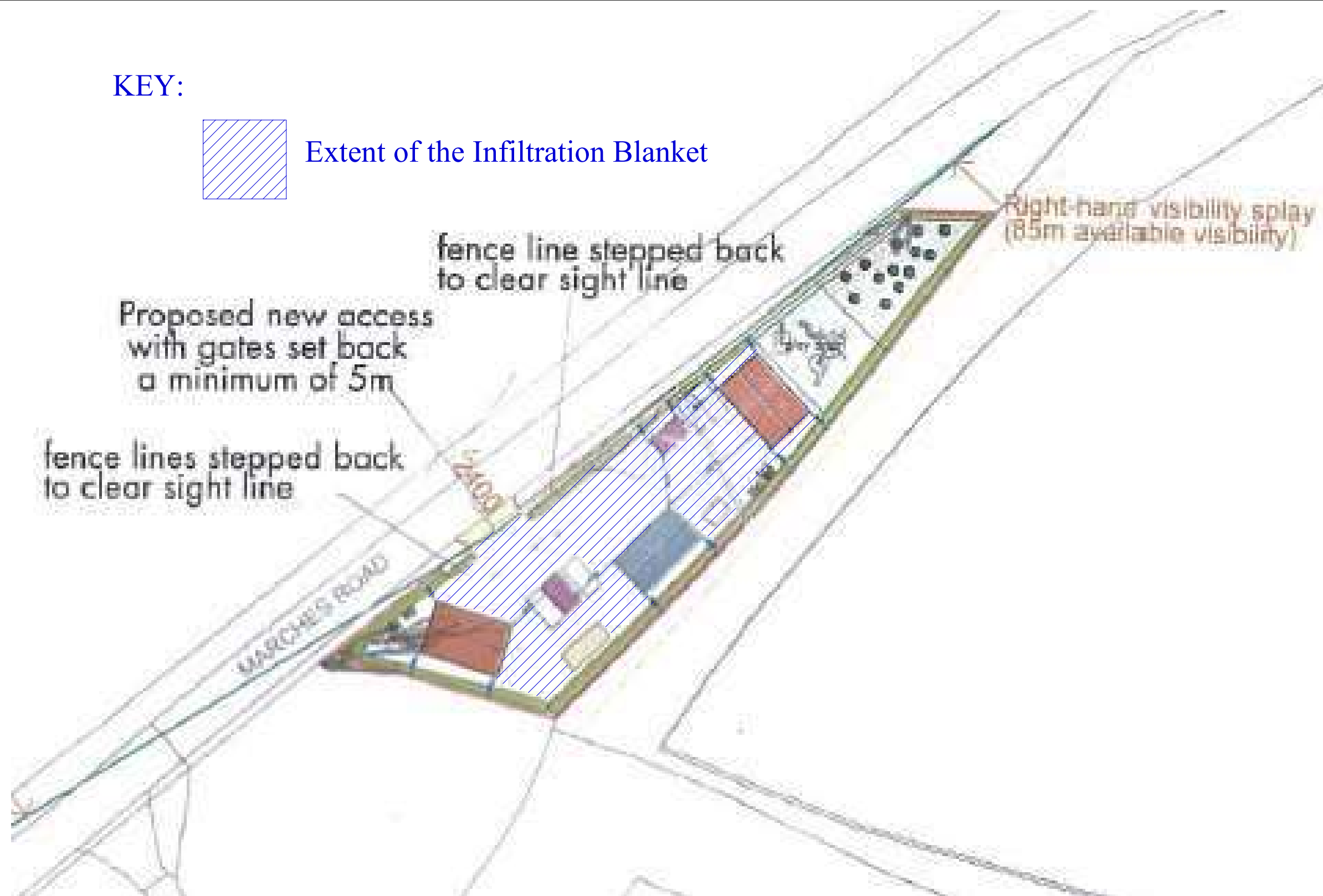
February 2026



KEY:



Extent of the Infiltration Blanket



0 5 10 15 20 25
metres



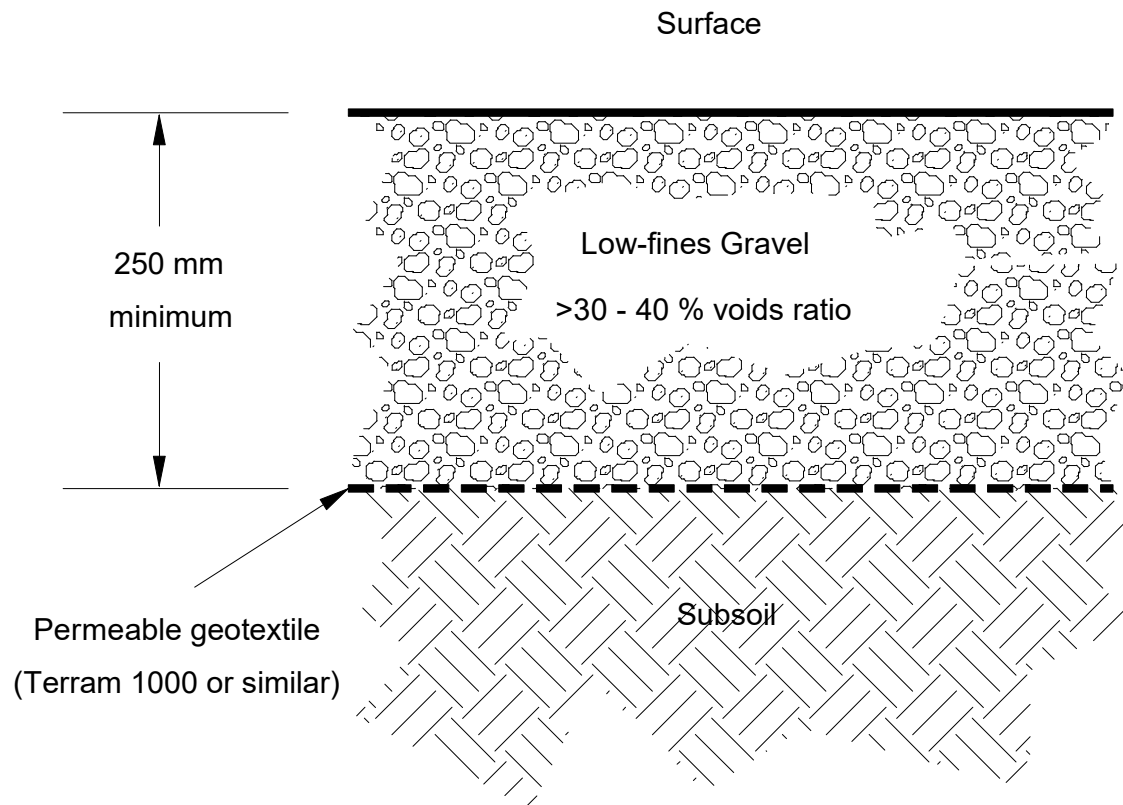
MR JOE MALONEY

SITE AT MARCHES ROAD, WARNHAM, WEST SUSSEX, RH12 3SE

Figure 3: Proposed Drainage Arrangement

Scale as shown

February 2026



MR JOE MALONEY

Scale as shown

SITE AT MARCHES ROAD, WARNHAM, WEST SUSSEX, RH12 3SE

Figure 4: Infiltration Blanket Detail

February 2026

APPENDIX B **Site Photographs**



Photo 1: Test Hole 1.

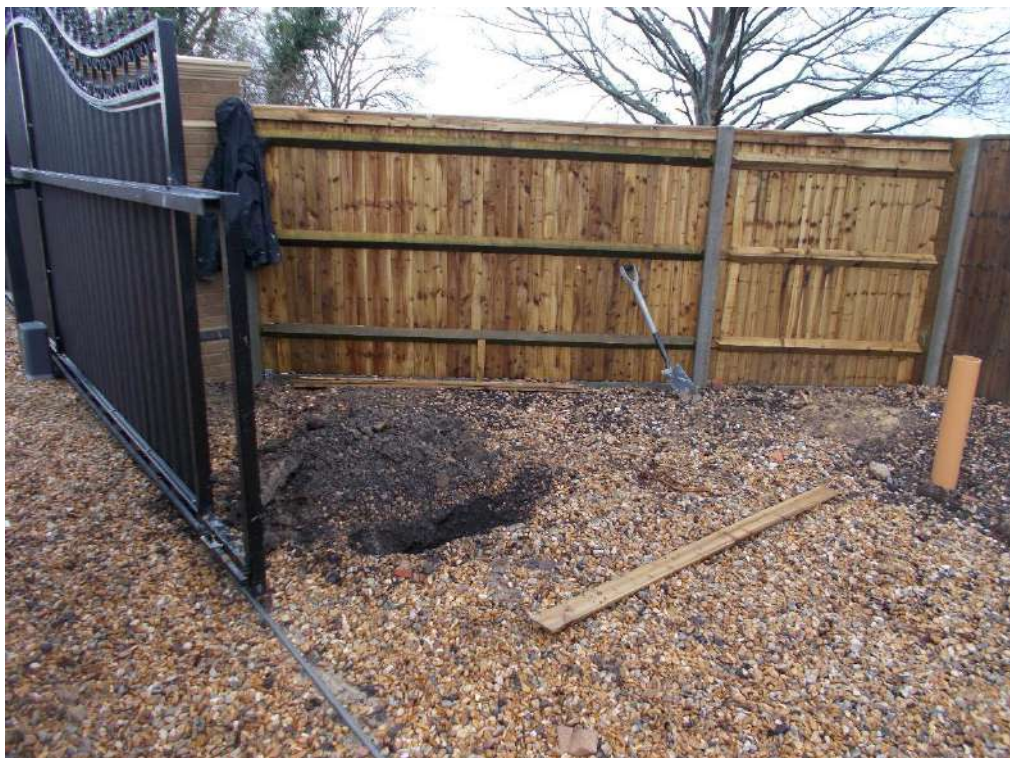


Photo 2: Test Hole 2



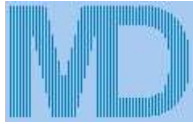
Photo 3: Test Hole 3



Photo 4: Degraded ditch in hedgerow on opposite side of Marches Road.

APPENDIX C

MasterDrain Model Output



MasterDrain
SW 16.56

Environmental Assessment Services Ltd

<http://www.easitd.co.uk>

London Rd, Hickstead
Haywards Heath,
West Sussex, RH17 5LZ
Tel: 01444 882552
email: info@easitd.co.uk

Job No. 875		
Sheet no. 1		
Date 09/02/26		
By MDM	Checked	Reviewed

Project Site at Marches Road, Horsham	
Title Blanket soakaway calculations for HORSHAM	

Data:-

Location hydrological data (FSR):-

Location	= HORSHAM	Grid reference	= TQ1730
M5-60 (mm)	= 26.2	r	= 0.34
Soil index	= 0.45	SAAR (mm/yr)	= 800
WRAP	= 4	Area	= England and Wales

Soil classification for WRAP type 4
Clayey, or loamy over clayey soils with an impermeable layer at shallow depth.

Design data:-

Safety factor = 1.5 - No damage or inconvenience (SF=1.5)
Fill porosity = 0.35 - Uniform gravel (porosity = 0.3 - 0.4)

Equivalent porosity (n1) = 0.35

Area drained = 100 m²

Infiltration coefficient = 0.007884 m/hr
Effective inf.coeff (q) = 0.005256

Return period = 100 yrs

Climate change factor = 45%

Calculations :-

Hmax = (Duration of storm/porosity)*([AreaDrained/(ExWidth*ExLen)} x Rain. depth/1000]-Inf.coeff)
Note: Duration of storm in hours, Rainfall depth in mm/hr x Climate Change factor.

Results :-

Emptying time to 50% volume = 8:31 (hr:min)

hMax (Depth) = 0.26 metres

Time to maximum = 0:05 hr:min

Rainfall at maximum = 22.04mm/hr

Width (m) = 10.0

Length (m) = 10.0

Total Infiltration area = 110.2m² (base area + sidewall area).

Total available volume = 8.95m³

N.B. The rainfall rates are calculated using the location specific values above in accordance with the Wallingford procedure.
Formulae and methods from CIRIA 156.

APPENDIX D
**UK Sustainable Drainage (UKSUDS) Calculations (Greenfield Runoff Rate and
Required Storage Volumes)**

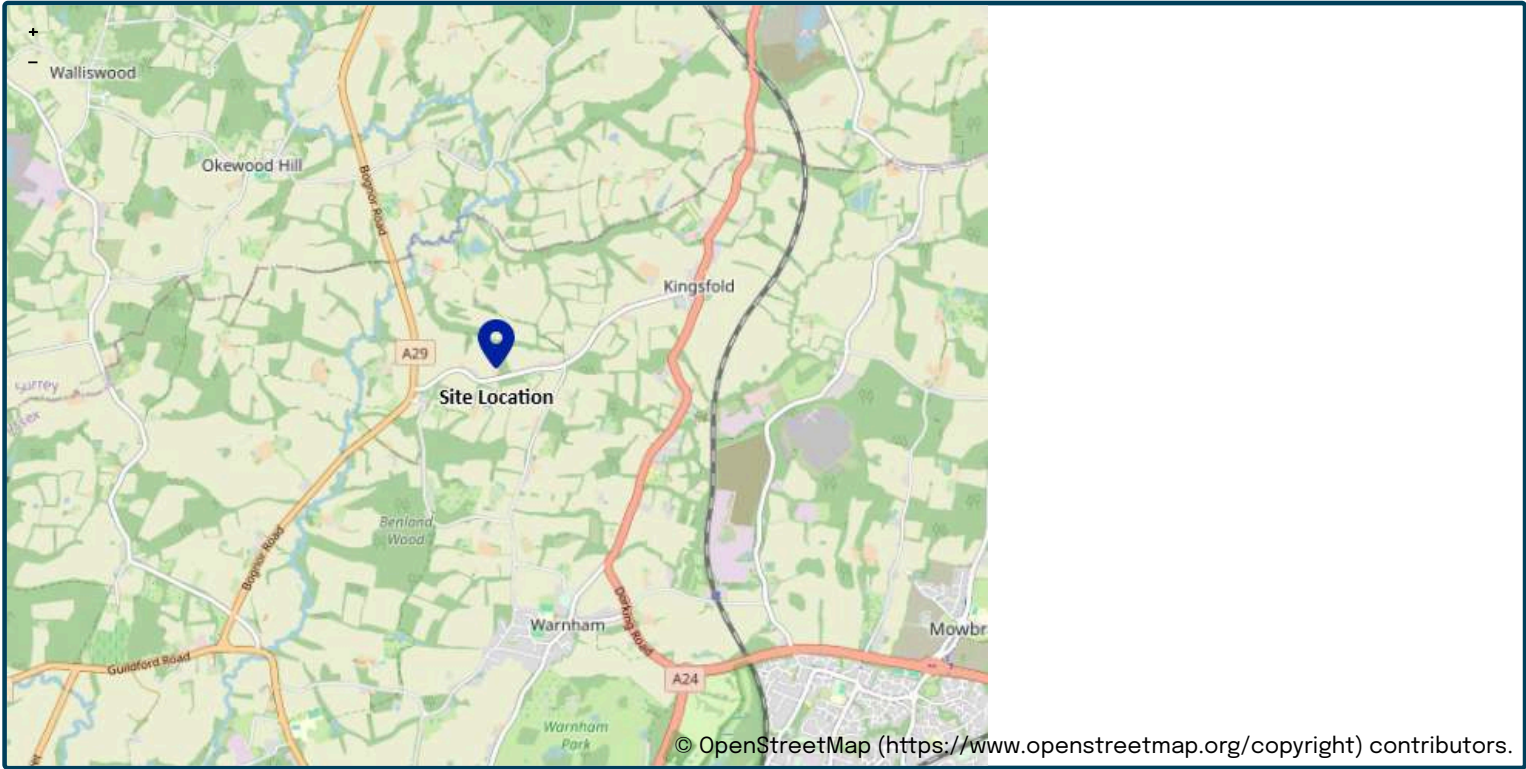
This is an estimation of the greenfield runoff rates that are used to meet normal best practice criteria in line with Environment Agency guidance “Rainfall runoff management for developments”, SC030219 (2013), the SuDS Manual C753 (CIRIA, 2015) and the non-statutory standards for SuDS (Defra, 2015). This information on greenfield runoff rates may be the basis for setting consents for the drainage of surface water runoff from sites.

Project details

Date	06/02/2026
Calculated by	Malcolm McKemey
Reference	882
Model version	2.2.2

Location

Site name	Land at Marches Road
Site location	Warnham



Site easting (British National Grid)	515250
Site northing (British National Grid)	135595

Site details

Total site area (ha)	0.15	ha
----------------------	-----------------------------------	----

Greenfield runoff

Method

Method	IH124
--------	-------

IH124

	<u>My value</u>		<u>Map value</u>
SAAR (mm)	789	mm	789
How should SPR be derived?	WRAP soil type		
WRAP soil type	4		4
SPR	0.47		
QBar (IH124) (l/s)	0.8	l/s	

Growth curve factors

	<u>My value</u>		<u>Map value</u>
Hydrological region	7		7
1 year growth factor	0.85		
2 year growth factor	0.88		
10 year growth factor	1.62		
30 year growth factor	2.3		
100 year growth factor	3.19		
200 year growth factor	3.74		

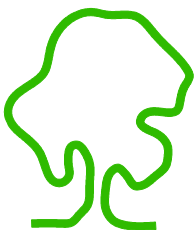
Results

Method	IH124	
Flow rate 1 year (l/s)	0.7	l/s
Flow rate 2 year (l/s)	0.7	l/s
Flow rate 10 years (l/s)	1.3	l/s
Flow rate 30 years (l/s)	1.9	l/s
Flow rate 100 years (l/s)	2.7	l/s
Flow rate 200 years (l/s)	3.1	l/s

Please note runoff estimation is subject to significant uncertainty. Results are therefore normally reported to only 1 decimal place. Where 2 decimal places are provided, this does not indicate accuracy to this level, it has been adopted to prevent ‘zero’ figures from being reported. Outputs less than 0.01 l/s are reported as 0.01 l/s.

Disclaimer

This report was produced using the Greenfield runoff rate estimation tool (2.2.2) developed by HR Wallingford and available at [uksuds.com](https://www.uksuds.com) (<https://www.uksuds.com/>). The use of this tool is subject to the UK SuDS terms and conditions and licence agreement, which can both be found at [uksuds.com/terms-conditions](https://www.uksuds.com/terms-conditions) (<https://www.uksuds.com/terms-conditions>). The outputs from this tool have been used to estimate Greenfield runoff rates. The use of these results is the responsibility of the users of this tool. No liability will be accepted by HR Wallingford, the Environment Agency, Centre for Ecology and Hydrology, Wallingford Hydrosolutions or any other organisation for the use of these data in the design or operational characteristics of any drainage scheme.



eas ltd

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Phone : +44 (0) 1444 882552

email : info@easltd.co.uk

web site : www.easltd.co.uk

Ref: DC/25/0982
14th August 2025

Recipient Address:

Horsham District Council – Planning Dept

FAO: Kate Turner

**Provision of further information requested REGARDING:
DISPOSAL OF SEWAGE**

Ref: DC/25/0982

Land at Marches Road Warnham

Dear Kate,

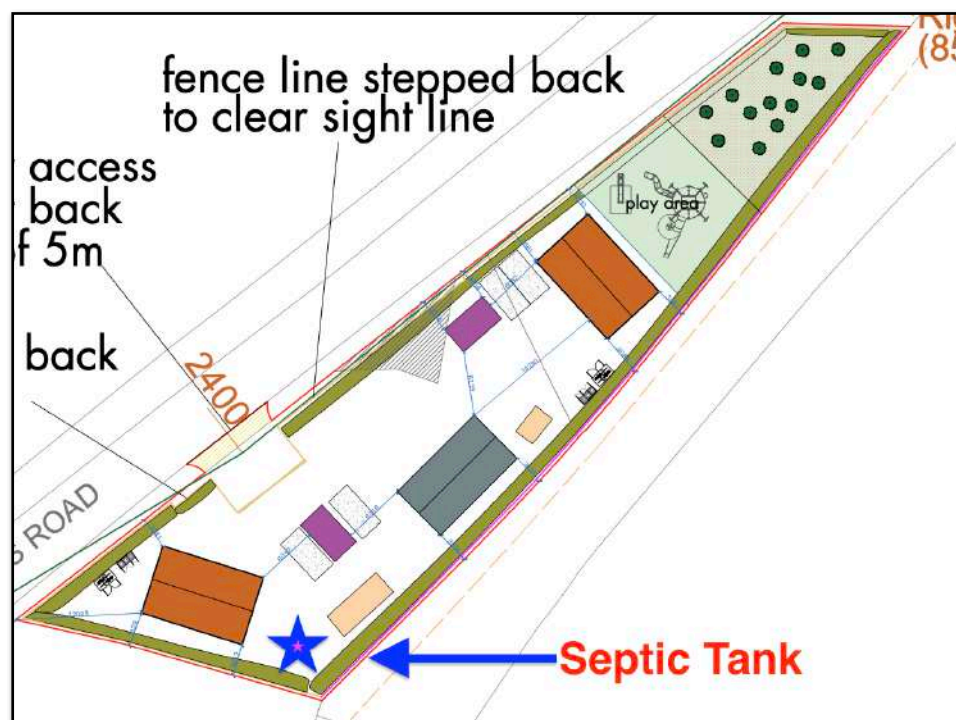
As requested by Penny Marsh, in letter dated 12th August 2025, I am providing further information in line with my client's application:

DC/25/0982 - Land at Marches Road Warnham

Please see below extracts, which detail the site's means of disposing and treating it's sewage.

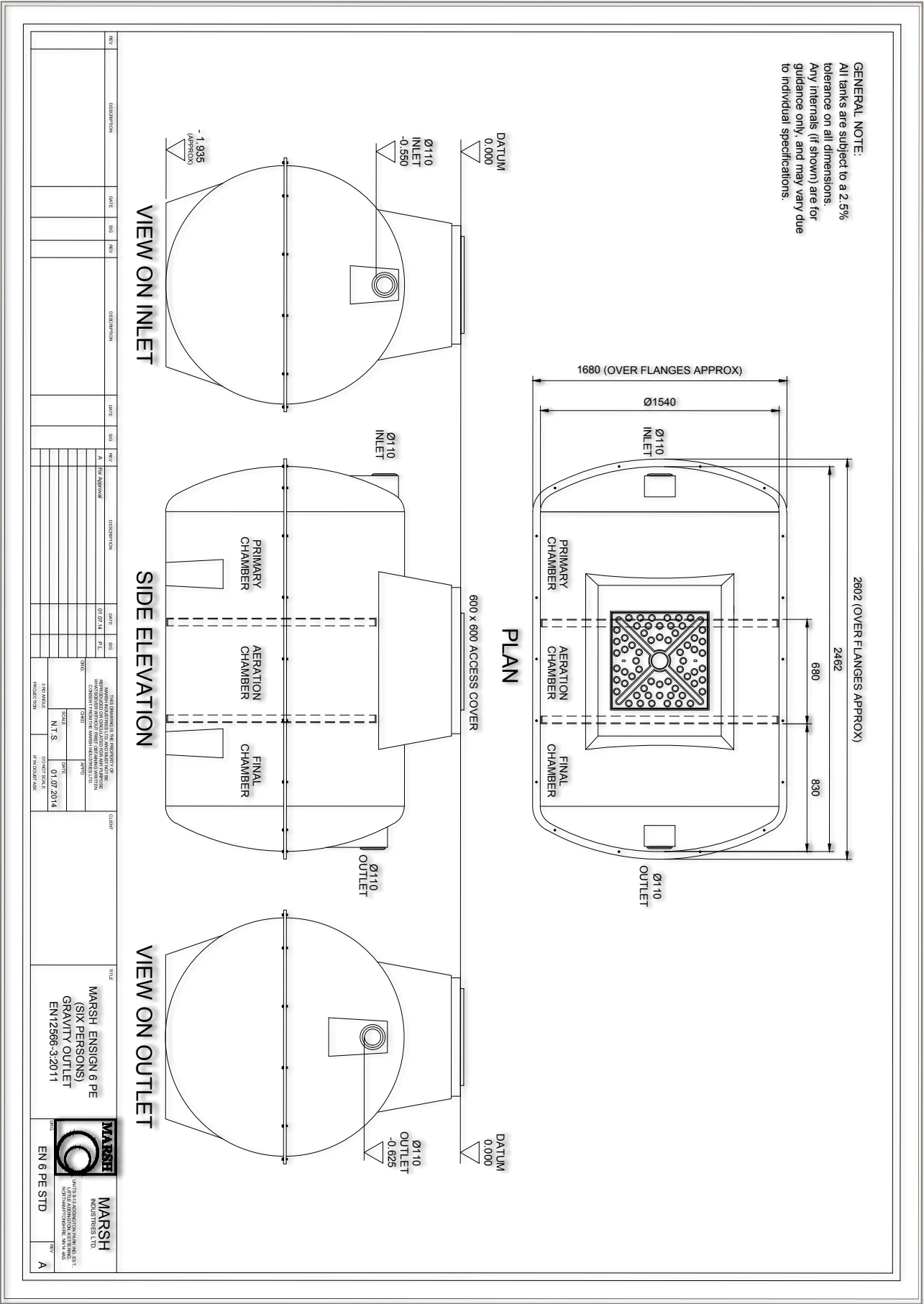
A septic tank system has been installed on site to safely manage all sewage generated by the property. Foul waste from toilets, sinks, and other wastewater outlets is directed via a sealed drainage pipe into the underground septic tank. It is located to allow for safe maintenance, desludging, and inspection. By isolating, containing, and partially treating sewage on site, the system ensures effective foul drainage management in an area without mains sewer connection, protecting public health and the local environment.

Location of The Septic Tank:



As displayed on the plan, the Septic Tank is placed in the South-Eastern corner of the property.

The specifications of the tank are shown below:



We trust that the information/detail submitted, is sufficient to negate any concerns surrounding sewage treatment and disposal on-site.

Kind regards,

