



COBWOOD SOLAR FARM

West Grinstead

CONSTRUCTION METHOD STATEMENT - WATER

Project No	25-387-70
Revision No	02
Issue date	22/07/26

Control Sheet

This report has been prepared by Calibro Consultants Ltd for the sole benefit and use of the Client. Calibro Consultants Ltd offer no liability for the information contained within the report to any third party.

Prepared by	Signature	Date
Alex Bearne BSc MSc Associate Flood Risk Consultant & Hydrologist		22/07/2026

Reviewed by	Signature	Date
Patrick Goodey BSc MSc Head of Flood Risk & Hydrology		22/07/2026

Approved for issue by	Signature	Date
Patrick Goodey BSc MSc Head of Flood Risk & Hydrology		22/07/2026

CONTENTS

1	BACKGROUND	1
2	SITE DESCRIPTION	2
3	SITE PROPOSALS	7
4	CONSTRUCTION METHOD STATEMENT - WATER	8
5	SUMMARY	15

Figures

Figure 2-1	Site Location	2
Figure 2-2	Solar Parcels.....	3
Figure 2-3	Topography and Hydrology	4
Figure 2-4	BGS Bedrock Deposits	5

Appendices

Appendix A Site Proposals

1 BACKGROUND

1.1.1 This report has been prepared to support the discharge of part of Condition 8 (construction method statement - water) as listed in the Decision Notice from Horsham District Council dated 21/05/2024 (application reference DC/23/2172).

1.1.2 Condition 8 states:

'Prior to the commencement of development, construction drawings of the surface water drainage network, associated sustainable drainage components and flow control mechanisms and a construction method statement shall be submitted and agreed in writing by the local planning authority. The scheme shall then be constructed as per the agreed drawings, method statement, FRA (07/03/24) and Response to LLFA comments dated February 2024 and Drawing Preliminary Drainage Strategy by PFA Consulting, rev A, dated 07/03.24 and remain in perpetuity for the lifetime of the development unless agreed in writing by the Local Planning Authority. No alteration to the agreed drainage scheme shall occur without prior written approval from the Local Authority.'

1.1.3 The remaining parts of the condition wording are addressed by the Drainage Conditions Discharge Reporting, document reference 25-387-70-030 and provided elsewhere in the condition discharge documents.

1.1.4 A full method statement should identify the roles and responsibilities, including contact names and telephone numbers of the contractor and personnel responsible for fulfilling its requirements. At the time of writing, the contractor is yet to be procured and therefore this information is not available.

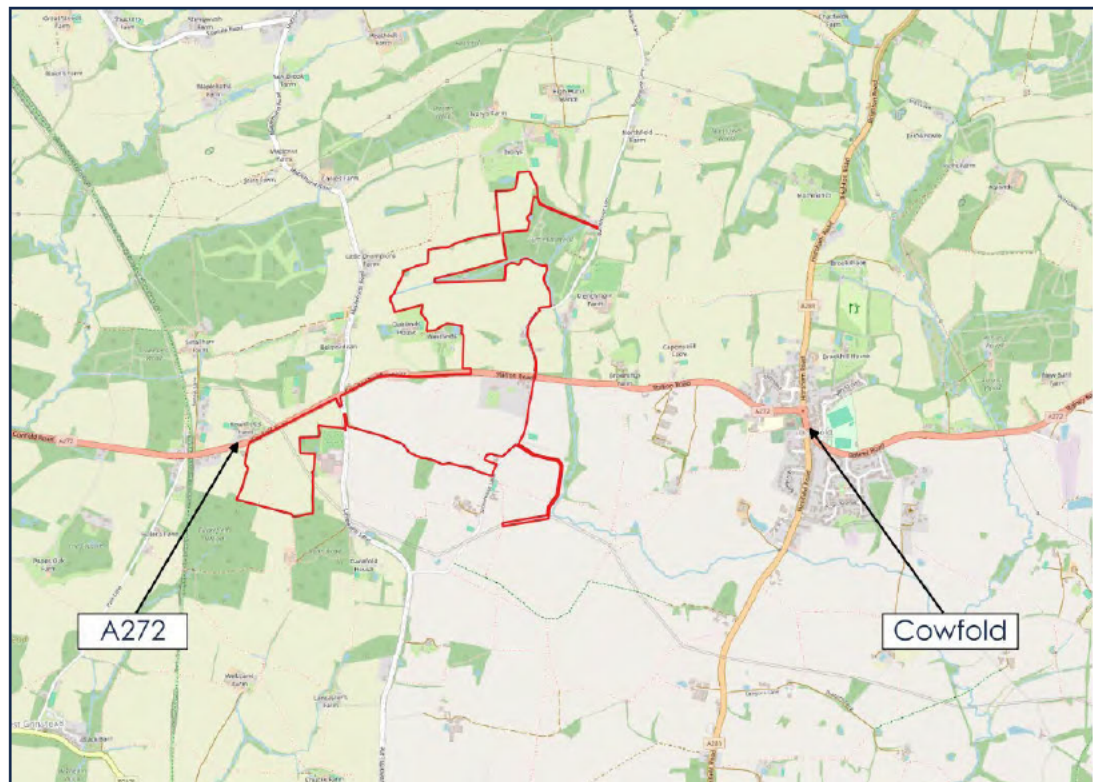
1.1.5 Therefore, this method statement is in outline form and provides the general arrangements and activities to be adopted and minimise impact to the water environment. The final details will be confirmed in the detailed method statement when the preferred contractor is appointed and full construction methodology confirmed.

2 SITE DESCRIPTION

2.1 Site Location and Setting

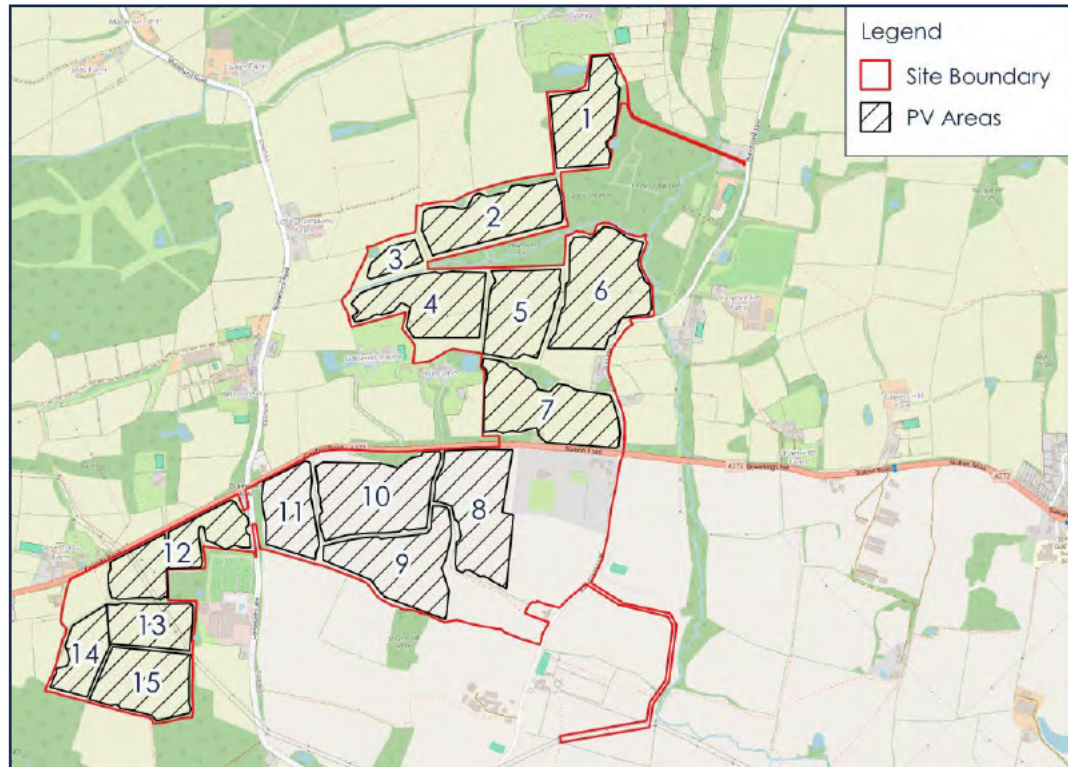
- 2.1.1 The site is approximately 79 hectares (ha) and is located approximately 1km from Cowfold and is bisected by the A272 (Cowfold Road / Station Road).
- 2.1.2 The approximate coordinates for the centre of the site are 519550, 122850. The nearest postcode is RH13 8LZ.
- 2.1.3 Figure 2-1 shows the site location.

Figure 2-1 Site Location



- 2.1.4 The existing site is greenfield, comprising almost entirely arable agricultural land. The site is separated into 15 numbered parcels. These parcels are shown in Figure 2-2 below.

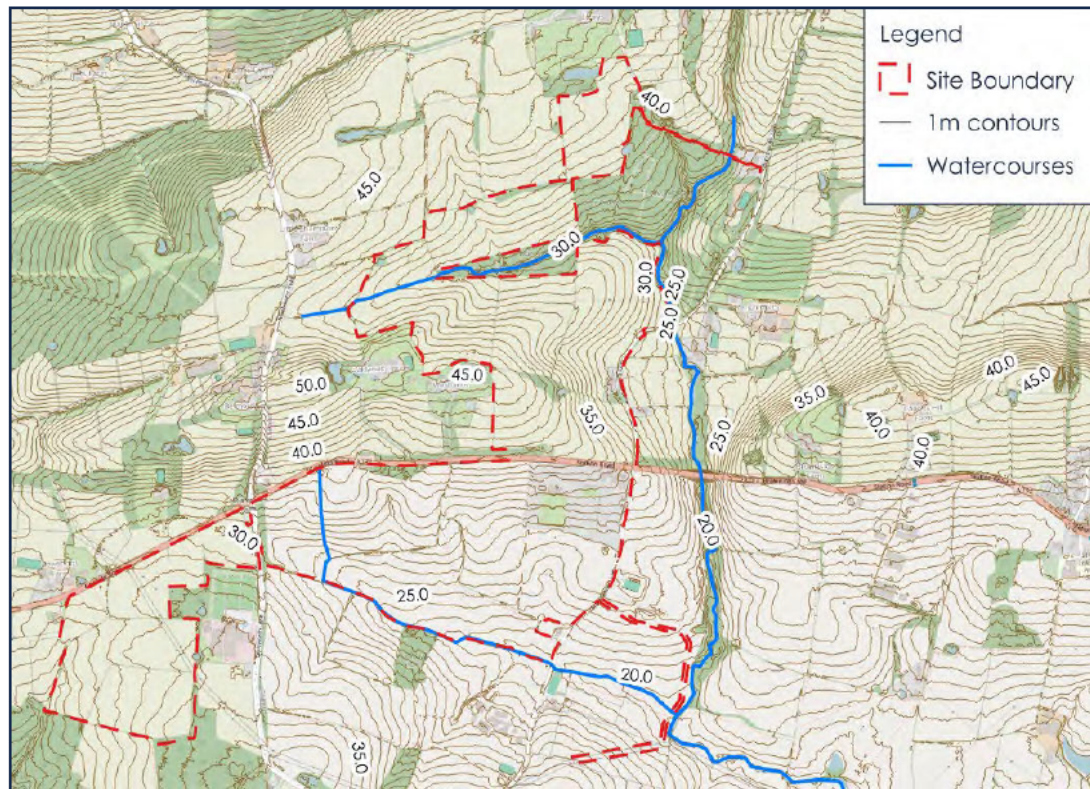
Figure 2-2 Solar Parcels



2.2 Topography and Hydrology

- 2.2.1 The site is elevated above most of the surrounding land. Within the site, three watercourses are shown in Ordnance Survey data and are evident in LiDAR data. Browning's Gill flows from north to the south. The other two watercourses flow into Browning's Gill from the west. The watercourses are shown along with 1m contours derived from LiDAR data in Figure 2-3.
- 2.2.2 The layout has been derived to preserve a 3m buffer from these watercourses and from field ditches. The swales are also located at least 1m from site fences.

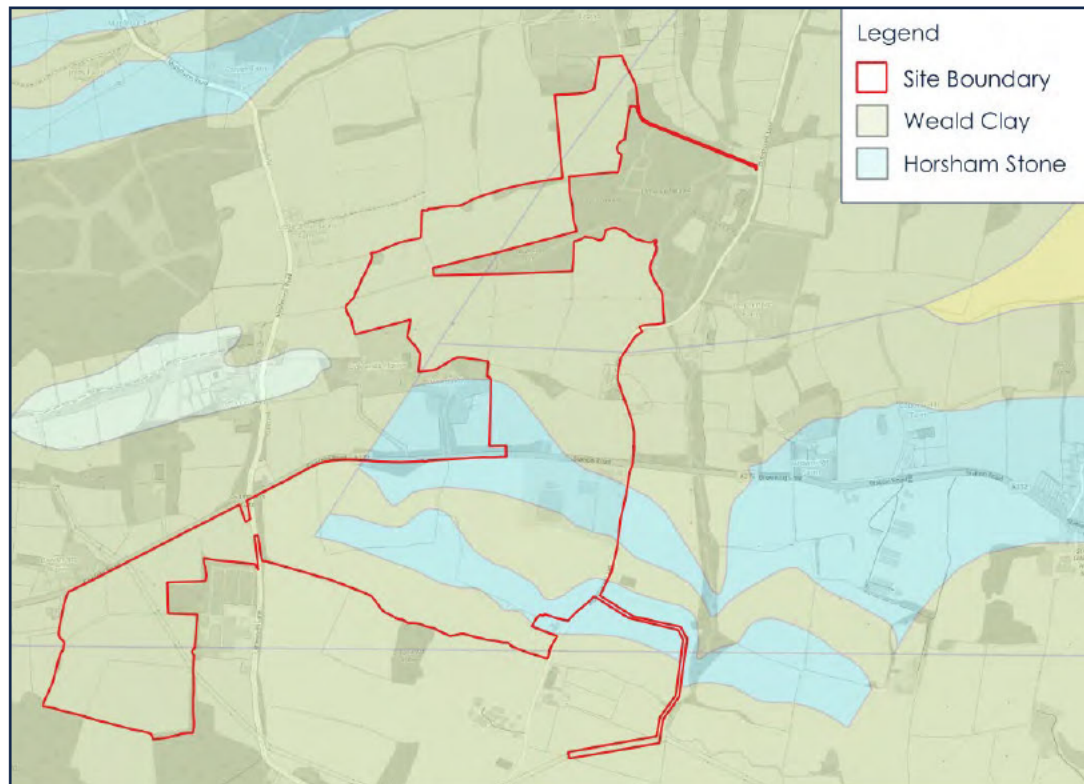
Figure 2-3 Topography and Hydrology



2.3 Geology and Soils

- 2.3.1 British Geological Survey (BGS) data shows the majority of the site to be underlain by 'Weald Clay Formation-Mudstone' which is essentially impermeable. There are also two bands of 'Horsham Stone Member-Sandstone' which is also essentially impermeable. BGS bedrock deposits are shown in Figure 2-4.
- 2.3.2 No superficial deposits are recorded by BGS at the site.
- 2.3.3 The BGS hydrogeology dataset records the entire site as being underlain by 'Rocks with essentially no groundwater'.
- 2.3.4 Soils mapping shows the site to be underlain by 'Slowly permeable seasonally wet slightly acid but base-rich loamy and clayey soils' with 'impeded drainage'.
- 2.3.5 The BGS Parent Material European Soil Bureau Description dataset shows that the soil profile matches the superficial geology in that the soils above the Weald Clay formation are clay to clayey loam.
- 2.3.6 The clay to clayey loams are denoted as being 'deep', which is the deepest classification demonstrating the depth of the soil layer is at least 1m. The sand to sandy loam soils are Deep to Intermediate, meaning they can '....be easily dug to a depth of 1m, sometimes more in places'.

Figure 2-4BGS Bedrock Deposits



2.4 BGS Boreholes

- 2.4.1 There is one published borehole¹ within the vicinity of the site, located to the north of the A272 and the westernmost part of the site. This borehole corroborates the BGS data in that the upper horizons are clay dominated. The interbedded sandstone and mudstone bedrock was encountered approximately 46mbgl (below) ground.
- 2.4.2 The borehole struck water at 45m below ground level (bgl). This shows that there is a significant unsaturated zone between the site and the groundwater. The groundwater rose to 8m bgl before test pumping commenced. This demonstrates the groundwater is confined by impermeable upper strata.

2.5 Designations

- 2.5.1 MAGIC mapping shows there to be no bedrock aquifer designation associated with the Weald Clay mudstone but the Horsham Stone is a Secondary A Aquifer, meaning it '*...can support local water supplies and may form an important source of base flow to rivers*'. Regarding the second part of the classification, borehole data shows that the rivers are perched above groundwater, meaning this classification would not apply to the site.

¹ <https://api.bgs.ac.uk/sobi-scans/v1/borehole/scans/items/21565186>

- 2.5.2 MAGIC mapping shows no aquifer designation associated with the superficial geology.
- 2.5.3 Regarding groundwater vulnerability, MAGIC mapping also shows the Weald Clay to be unproductive but Horsham Stone to have a High vulnerability.
- 2.5.4 MAGIC mapping shows the site to fall outside any Source Protection Zones or Drinking Water Safeguard Zone for surface water or groundwater.
- 2.5.5 There are no known Sites of Special Scientific Interest (SSSIs) within the vicinity of the site.

3 SITE PROPOSALS

- 3.1.1 Planning permission has been obtained to develop land presently comprising arable farm land to a solar farm.
- 3.1.2 The predominant infrastructure on the site will be the solar panels themselves which will be mounted on metal frames anchored to the ground via supports driven into the ground.
- 3.1.3 There will be isolated inverters dispersed across the site. These would be containerised in shipping container style units. They would be laid on gravel bases excavated into the ground and serviced by a network of buried cables.
- 3.1.4 Additional buried cables would be laid across the site, within backfilled trenches of approximately 1m depth and width.
- 3.1.5 The cabling will include CCTV cables serving cameras, and lighting on steel poles.
- 3.1.6 Two DNO substations and two customer switchgear station are proposed on the site which would also be containerised and laid on a gravel base.
- 3.1.7 The site will be accessed from the existing highways via a network of access tracks, formed from permeable granular material constructed by removal of approximately 300mm of topsoil to be replaced with the granular material.
- 3.1.8 Where watercourses are to be crossed, they would utilise existing crossings. It is assumed by this report that cables would be attached to existing crossings.
- 3.1.9 The site will have perimeter fencing up to 2.m high.
- 3.1.10 As part of the surface water management part of the proposals, numerous swales are proposed around the perimeter of the generation areas. These will assist with the interception of overland flows, encouraging water to percolate to the ground.
- 3.1.11 The proposals include three construction compounds. These compounds will either be formed of permeable stone over a permeable geotextile or using temporary matting. Site proposals are provided in Appendix A of this report.

4 CONSTRUCTION METHOD STATEMENT - WATER

4.1 Construction Environmental Management Plan

- 4.1.1 A CEMP focussing on biodiversity is to be submitted to discharge Condition 4. This will cover general site operations and mitigation to minimise impacts on the environment.
- 4.1.2 Consequently, this report focusses on the potential activities that could impact the water environment and proposes mitigation where required to minimise the likely impact on surface water and groundwater.

4.2 Panel Installation

Panel Supports

- 4.2.1 Panel installation would be the most significant construction activity across the site.
- 4.2.2 The activities will be completed in accordance with the Environment Agency's (EA) Pollution Prevention Guidelines and in particular GPP5: Works and Maintenance in or near Water and GPP6: Working at Construction and Demolition Sites.
- 4.2.3 The panel supports are very likely to be driven into the ground to a depth of approximately 1 - 1.5m. The method means that there would be no need for concrete foundations and the arisings would be negligible.
- 4.2.4 The site has deep clayey soils and groundwater is expected to be many metres below the ground (see Section 2.3 & 2.4). Consequently, they are likely to remain within the soil horizon across the site and comfortably above the likely groundwater level, leaving a significant unsaturated zone between the supports and the groundwater. The prevalence of high proportions of clay within the site soils is likely to restrict the vertical migration of water.
- 4.2.5 As a result, the proposed supports would not introduce a significant pathway for the mobilisation of sediment or contaminants to groundwater and it is concluded that this risk can be scoped out of further assessment or mitigation consideration.

Vehicle Movement and Compaction

- 4.2.6 Without mitigation, the movement of vehicles across the site has the potential to cause compaction and an associated increase in runoff and soil erosion. This is a potential issue particularly on more steeply sloping parts of the site and given the high clay content of the soil profile meaning that percolation may be slightly restricted.
- 4.2.7 There are only a few watercourses around and through the site, with the larger, such as the Browning's Gill, being significantly set back from the proposals. Therefore, a

considerable buffer for runoff already exists to the larger watercourses, which would limit the pathway for entry of sediment.

4.2.8 The following measure are proposed to minimise compaction and runoff:

- Construct and utilise access tracks, construction compounds and lay down areas early in the construction programme;
- Install laydown areas away from watercourses as far as practicable;
- Excavate swales early in the construction programme so they can capture runoff and sediment – reprofile swales at the conclusion of works to ensure the design capacity is realised in advance of operation;
- Use low ground pressure machinery to reduce compaction;
- Programme works for drier months, wherever possible and in all cases avoid working with heavy machinery in heavy rain / saturated soil conditions;
- Remain at least 3m from the top of watercourse banks, with the exception of watercourse crossings;
- Utilise or improve existing watercourse crossings;
- Silt fences (or equivalent silt protection means) to be installed to: at the base of steep slopes (i.e. > 1 in 10), to intercept overland flow towards watercourses and around topsoil/subsoils/spoil storage to prevent sediment runoff and surface water resource pollution;
- Where soil is disturbed, use tilling (mechanical loosening) to an appropriate depth and reseed using a native seed mix to promote ground cover and break up compacted soils.

4.3 Excavation and Groundworks

- 4.3.1 All works including excavations will be completed in accordance with BS 6031:2009 Code of Practice for Earthworks and in accordance with the EA's Pollution Prevention Guidelines and in particular GPP5: Works and Maintenance in or near Water and GPP6: Working at Construction and Demolition Sites, as per the panel installation.
- 4.3.2 The isolated inverters, switchgear and substations would be sited on concrete plinths or pads above a gravel base. The gravel bases would likely extend to a depth of approximately 0.3m below the ground level, consequently only needing similar depths of excavation.
- 4.3.3 The drainage strategy for the site comprises a number of grassed swales with a minimum depth of 0.3m, as detailed in the Drainage Condition Discharge Report, document reference 25-387-70-030-03.

- 4.3.4 The deepest excavations on the site would therefore likely be the laying of cables across the site, which would be in trenches up to approximately 1m deep. These would be backfilled with the excavated material and re-turfed.
- 4.3.5 As with piling activities, excavations and groundworks would most likely remain within the soil horizon and comfortably above the groundwater and therefore unlikely to create a direct pathway to the aquifers.
- 4.3.6 Construction would ideally be programmed to be completed in dry conditions. In addition, groundwater is likely to be comfortably below the base of excavations and at a minimum when the land is not at field capacity, restricting the chance for groundwater being encountered during excavation. It is therefore very unlikely that dewatering would be required even for the deepest proposed excavations, which are only to 1m. Any proposed dewatering activities would comply with the Environment Agency's Regulatory Position Statement 261 – Temporary dewatering from excavations to surface water.
- 4.3.7 The most significant source of sediment in runoff would be from exposed ground or arisings from the isolated infrastructure excavations. Given the isolated and dispersed nature of these, arisings or exposed ground would not be extensive. The exposed ground would be caused by excavating the container bases and therefore rainfall would naturally sit within the excavation without creating a pathway for overland flow.
- 4.3.8 General silt control measures would be sufficient to manage the arisings from these excavations and negate the chance for release of sediment into the ground or via overland flow. Measures would include:
- No excavations within 3m of a watercourse, with the exception of crossings;
 - Arisings to be stored on impermeable liners to prevent leaching to the ground;
 - Arisings to be covered in impermeable liners to reduce washing;
 - Excavations to be undertaken when required and not in advance;
 - Surplus arisings to be re-used on site to avoid carting it away. For example, arisings from swale excavation could be used to form a low embankment on the downstream side of the swale and increase the capacity of the features.
- 4.3.9 The following measures would be taken to prevent contamination from chemicals and hazardous products
- Any chemicals or hazardous products will not be installed less than 50 m from a watercourse.
 - As far as practical, storage of chemicals shall be: 1) on secondary containments, and; 2) at least 50 m away from any watercourse.

4.4 Vehicular Watercourse Crossings

4.4.1 The majority of watercourse crossings, either vehicular or cable, would utilise existing crossings wherever possible. A review of the proposed layout suggests one new vehicular crossing is potentially required.

4.4.2 The detailed method statement for a new watercourse would be confirmed upon appointment of the preferred contractor and subject to the Ordinary Water consenting process, which would be completed post-condition discharge.

- 4.4.3 However, the crossing methodology would include the following general activities:
- Utilise existing crossings as a priority. New crossing types to be span bridges or, if these are possible then box culverts or, if these are not possible then culverts;
 - Crossing type and diameter to be confirmed during the consenting process;
 - Where a dry working is required, e.g. to lay a culvert bedding, dam the watercourse either side of the crossing location (upstream first) and provide over-pumping to ensure continuation of flow;
 - If pumping watercourse water, install adequate protection system to protect fauna during pumping operations (e.g. meshing system at pumping inlet);
 - Programme works to occur during dry weather, wherever possible;
 - Install hay bales or silt fences upstream and downstream of the crossing area to capture silts released;
 - Minimise bank disturbance but focussing groundworks to the crossing area only;
 - Minimise the storage of materials or fuels in the working area;
 - Use of drip trays, nappies or similar underneath pumps or stored fuels;
 - Locate stored materials as far from the watercourse as possible. Insofar as practicable, inert materials to be stored at least 8m and hazardous materials at least 50m from the top of bank of watercourse.
 - Use impermeable membranes to store excavated material, where this material is to be reinstated. Otherwise re-use excavated material on-site to minimise cart away;
 - Protect topsoil storage against any possible loss (mixing with subsoil or sediment run off) or contamination
 - Retain disturbed bed substrate, bank vegetation or similar to return the bed and bank to its original profile and cover; or
 - The reprofiled watercourse would be re-turfed, seeded or planted with a native mix. A seed impregnated mat may be required to minimise erosion of exposed banks (if re-turfing is not possible) if works are conducted outside of growing season.

4.5 Cable Watercourse Crossings

- 4.5.1 The detailed cable routing through the site is yet to be confirmed. Cables crossing watercourses would be kept to a minimum and where they need to do so, they would utilise existing crossings wherever possible.
- 4.5.2 Where cables need to cross watercourses and there is no existing crossing, they could potentially use two methods, open-cut trenching or horizontal directional drilling (HDD).

Open-Cut Trenching

- 4.5.3 Regarding the open trenching, the risk of pollution is limited to accidental spillages of hydrocarbons or release of sediment, which is highly unlikely and mitigated by ensuring plant and equipment is well maintained and regularly inspected.
- 4.5.4 Regardless, spill kits would be provided and carried with the trenching plant to ensure rapid clean up in the very unlikely event of spillage.
- 4.5.5 Excavated material would only be stockpiled for very short periods as it would quickly be used as backfill to return the watercourse bed profile. Depending on weather conditions, such material may be placed on impermeable mats or sheets. Due to the quick turnaround from excavation to backfill, impermeable covers would only be required to prevent washout during heavy rainfall.
- 4.5.6 Suppression spraying may be required to minimise the airborne spread of dust. However, the volumes of dust would be low, meaning it would settle local to the works without causing noticeable settlement of sediment in the watercourse.
- 4.5.7 Open trenching across watercourses would be conducted during dry periods, wherever possible, to minimise the chance for low impediment.
- 4.5.8 Where this is not possible, a dry section will be created by placing an earth bund first upstream then downstream of the trenching route. The bund would be covered in an impermeable liner, if required, to prevent soil washout. Dry conditions would be created by pumping water from the bunded trench route.
- 4.5.9 Where required, to ensure flow continuity, over pumping would be used to pass flow from upstream to downstream of the bunded section. If this is used, hay bales or silt fences will be deployed downstream of the pump discharge location to filter and settle sediments.
- 4.5.10 Regardless of the conditions on the day, the watercourse would be excavated with bank and bed material reserved for replacing post-installation.
- 4.5.11 The trench would be excavated to a depth that would provide at least 0.6m of cover between the cable and the watercourse bed, but ideally 1m depending on excavation depth and trench stability.
- 4.5.12 The watercourse would be restored to its existing dimensions and retained bed substrate returned as closely as possible. This would be completed prior to removal of the bunds.

- 4.5.13 The downstream bund would be removed first, followed by the upstream. Downstream hay bales and/or silt fences would either be retained (if used on account of wet conditions) or deployed. Then the upstream bund will be slowly removed to minimise sudden release of water. Finally silt fences or hay bales would be carefully removed.
- 4.5.14 It is likely the trenching through the watercourse would take less than one day and therefore the above measures would likely only be required in wet weather, hence the preference for scheduling the works during dry conditions.
- 4.5.15 The reprofiled watercourse would be re-turfed, seeded or planted with a native mix. A seed impregnated mat may be required to minimise erosion of exposed banks (if re-turfing is not possible) if works are conducted outside of growing season.
- 4.5.16 The above measures are subject to site setting out and the specific conditions identified at the site.

4.6 HDD

- 4.6.1 If deployed, HDD would be at a depth of at least 1.5m below the bed of the watercourse.
- 4.6.2 The proposed HDD methodology is relatively low-risk and would remain comfortably above the level of the groundwater, providing a significant unsaturated zone through low-permeability strata between the works and the groundwater, providing protection to the groundwater.
- 4.6.3 Outline HDD practices are outlined below:
- Ensure all operatives involved in the process are well trained and follow best practice and guidance such as BS EN 16228-3
 - Reflect known ground conditions to select a specific route and depth through the most homogeneous geological conditions possible;
 - Casing of weaker un-cohesive layers to reduce bentonite breakout;
 - Use as low a concentration of bentonite as reasonably practicable;
 - Best practice for HDD drilling mud managed to be followed (likely to comprise on site treatment and recycling to minimise quantities used and the final disposal for treatment;
 - Additives used in the HDD drilling mud shall be harmless to fauna, flora, soil, groundwater, and surface water
 - Operatives to monitor the drilling for evidence of breakout and cease drilling and seal fissures or voids if applicable, as required;
 - Monitoring of drilling fluid returns and volumes to help identify losses;
 - Retain a stock of sandbags and pumps on site to contain breakout and dispose accordingly;
 - Locate launch and exit pits at least 8m from the top of watercourse banks;

- Utilise mains supplied water for bowzers;
 - Bentonite water slurry will be stored in the launch pits and transported to a registered disposal site(s); and,
 - HDD wastewater (including bentonite) will be incarcerated within the launch pit and transported to a specialised local facility for disposal.
- 4.6.4 A specific emergency response plan to be developed by HDD contractor for HDD crossings.
- 4.6.5 With the above measures in place, the potential for release of pollutants or sediment to surface waters or the groundwater would be negligible.

4.7 Spills and Leaks

- 4.7.1 The nature of the proposals means that there would be limited chance for a major leak of spillage. For example, it is unlikely that significant volumes of chemicals would need to be stored on the site.
- 4.7.2 All equipment / vehicles containing fuel or other hazardous materials will, where possible, be stored on hard standing (or other impermeable base) whilst not in use, be accompanied by a spill kit as necessary, and protected against vandalism.
- 4.7.3 Hazardous materials are to be limited, but where required, would be stored in bunded areas as a precaution.
- 4.7.4 Refuelling will take place on a designated area, with absorbent materials and drip trays to hand to prevent contamination. As far as practical, no refueling should take place within 50 m of a watercourse.
- 4.7.5 All necessary precautions will be taken to prevent the occurrence of smoke emissions or fumes from site plant or stored fuel oils.
- 4.7.6 Spill kits containing materials such as leak-sealing putty, over drums, drain seals, oil or chemical absorbents and personal protective equipment (PPE) will be located within and near storage and refuelling areas.
- 4.7.7 Emergency management protocols would be established to clarify the personnel and actions required to respond to an emergency such as a spillage of stored materials.

5 SUMMARY

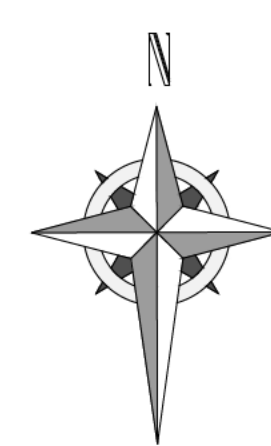
- 5.1.1 This report has been prepared to support the discharge of part of Condition 8 (construction method statement - water) as listed in the Decision Notice from Horsham District Council dated 21/05/2024 (application reference DC/23/2172).
- 5.1.2 The report presents an outline method statement in lieu of a contractor being appointed, who would provide a more detailed statement, including confirmation of roles, responsibilities and named parties overseeing construction activities.
- 5.1.3 The site proposals comprise the installation of solar generation equipment. The predominant infrastructure will be solar arrays, followed by a network of permeable access tracks. Containerised infrastructure such as inverters and substations will be isolated and dispersed across the site, limiting the amount of hardstanding proposed. Panels would be installed on supports, which would be driven to a depth of approximately 1 – 1.5m.
- 5.1.4 The containers would be sited on gravel bases approximately 0.3m deep and wrapped in a permeable textile. A number of swales approximately 0.3m deep would be installed at key locations around the perimeter of the generation area.
- 5.1.5 A network of cables would be installed across the site at depths of approximately 1m and would be backfilled using the excavated material and re-turfed.
- 5.1.6 The above demonstrates works will be relatively minor, with excavations being shallow, no deeper than approximately 1m.
- 5.1.7 There are few watercourses within the vicinity of the site, notably the Browning's Gill. However, the larger watercourses would be set back from the development, providing a vegetated easement of no development on either bank. For example, the watercourse remains outside the redline boundary with the exception of the existing crossing.
- 5.1.8 Minor ditches flow through parts of the site, most of which are field perimeter ditches so again development would be set back from them.
- 5.1.9 Local data suggests that the soils have a clay content which could restrict the downward migration of water through the upper horizons. In addition, BGS boreholes show the groundwater to be relatively deep, 45m below the ground. This means an unsaturated zone would exist between the site excavations and the groundwater.
- 5.1.10 The site lies outside SPZs and drinking water protected areas and the bedrock is mostly unproductive or with very deep groundwater, meaning that the groundwater is not particularly sensitive to change at the site.
- 5.1.11 Construction activities would be relatively low impact with silt control and measures to restrict soil compaction being the predominant mitigation requirements.

- 5.1.12 The risk of accidental spills or leaks would be low and mitigated through appropriate storage of materials, provision of spill kits and good site management practises.
- 5.1.13 Watercourses crossings (vehicular or cable) are yet to be confirmed but would be avoided where possible.
- 5.1.14 Where crossings are required (one potential vehicular crossing is presumed based on the current site layout), they would utilise existing routes wherever possible.
- 5.1.15 New crossings would therefore be limited and their detailed method statement subject to the consenting process, which would commence post-condition discharge and when a contractor is appointed. Nonetheless, this document recommends general good practice methods to minimise impacts on the watercourses.
- 5.1.16 Cable watercourse crossings would again be avoided or utilise existing crossings.
- 5.1.17 New crossings would either utilise open cut trenching or HDD, the full details of which would be subject to the consenting process. General measures have been outlined to demonstrate that the impact on the watercourses would be negligible.

APPENDICES

APPENDIX A

Site Layout



LEGEND

- PROPOSED SITE
- ENVOY/CONSTRUCTION ACCESS
- CONSTRUCTION AND MAINTENANCE ACCESS
- SITE ACCESS
- PERIMETER FENCILINE
- UG CABLE ROUTE
- UG CABLE ROUTE
- UG CABLE ROUTE
- UG CABLE ROUTE
- UG CABLE ROUTE
- UG CABLE ROUTE
- FOOTPATH
- EXISTING VEGETATION
- NEGATIVE COMPOUND AREA

SITE INFRASTRUCTURE:

- SOLAR PANELS
- MOBILE PHONE (GPRS) TOWER
- INVERTER
- SWITCHER
- SECURITY GATE
- CCTV CAMERA
- END VIL SUBSTATION

STATUS

PLANNING

PROJECT

Cobwood

Solar Farm

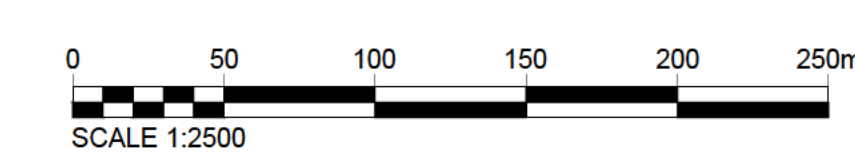


DRAWING TITLE

Cobwood

Site Layout External

Drawing: AE	Checked: NG	First issued: 18.11.2022
Project code: LCS049		Drawing Number: PLE-02
Sheet size: A0	Scale: 1:2500	Iteration: 17





📍 Spectrum | Bond St | Bristol | BS1 3LG

☎ 0117 2441 970

✉ hello@calibro-consultants.com

🌐 www.calibro-consultants.com

Registered office as above

Registered in England & Wales: 9988524



Transport Planning | Flood Risk & Hydrology | Infrastructure & Drainage

ITEMA Transforming the world
to sustainability