

***Woodview School, Ingfield Manor School,
Five Oaks, Billingshurst, RH14 9AX***

Salutem LD Bidco IV Ltd

GEOTECHNICAL GROUND INVESTIGATION REPORT

Project No. 2561-25



August, 2025

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Your ref:
Our ref: SKZ_2561/25

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SL4 1EG

REPORT ON GEOTECHNICAL GROUND INVESTIGATION AT WOODVIEW SCHOOL, INGFIELD MANOR SCHOOL, FIVE OAKS, BILLINGSHURST, RH14 9AX.

1. INTRODUCTION

AVZGeoEng Limited have been commissioned by Salutem LD Bidco IV Ltd ('the Client') to carry out a Geotechnical Investigation in regards to the proposed redevelopment at the above site as indicated by the client and in accordance with the AVZGeoEng quotation ref: SKZ_2561/25.

As we understood, the proposal is for erection of new SEN school and sports hall on the grounds to the South of the existing school, currently occupied by derelict former residential buildings.

2. THE SITE

The site, which may be found by approximate National Grid reference OS Grid Ref: E: 509197, N: 128820, is situated to the land with derelict buildings to the south of Woodview School, Ingfield Manor School, Five Oaks, Billingshurst, RH14 9AX.

An extract of the Ordnance Survey map showing the location of the site is included in **Figure 1**.

The published 1:50,000 scale geological map of the area indicates that the site is underlain by Weald Clay Formation - Mudstone as bedrock geology. Superficial Deposits are not indicated on the published Geological maps and records of the area.

Weald Clay Formation - Mudstone containing clay, dark grey thinly bedded mudstones (shales) and mudstones with subordinate siltstones, fine to medium grained sandstones, including calcareous sandstone (e.g. Horsham Stone Member), shelly limestones (the so called "Paludina Limestones") and clay ironstones.

The highest part of the Wealden, the Weald Clay Group, up to 400 m thick, probably formed at a time when the London-Brabant Massif was at its lowest. It contains a number of marine horizons, which record short lived breaching of the Weald mud-plains by the Boreal Sea (Gale, 2000). Detailed heavy

mineral provenance studies have shown that most of the detritus was derived from the Jurassic and to a lesser extent Palaeozoic rocks of the London Platform to the north. However, in the upper part of the Hastings Group, detritus originating from Cornubia spilled over from the Wessex Basin. The general abundance of detritus derived from the west, which includes material from the Iberian Peninsula, points to doming on the continental margin west of the UK during Early Cretaceous times.

The overlying Weald Clay has also been proved cyclic in nature, predominantly comprising grey shales and mudstones with intervening clastic rich layers representing periods of higher energy. The Weald Clay mudstones are characteristically bioturbated-laminated clays. There is common cyclicity throughout the clays. The fresh colour of the clays is characteristically a dark-medium green grey, and when weathered these changes to an orange ochre-brown type colour. There are bands of red mudstones associated with sandstone horizons and rootlet beds, diagnostic of periods of uplift to the surface.

The cyclicity and lithology changes outlined above may be attributed to environmental changes. The arenaceous formations, including sand and pebbly units is believed to have been deposited during periods of progradation of alluvial fans

The existing topography and history of development of the site suggest that in addition to these natural strata some made ground may be present on the site.

3. LIMITATIONS

Any opinions and interpretations expressed in this report are based on the ground conditions encountered during the site work and on the results of tests made in the field, together with AVZGeoEng conjectural interpretation between exploratory holes. There may, however, be other conditions prevailing at the site which have not been revealed by the investigation and which have therefore not been taken into account in this report. While the report may offer opinions on the possible configuration of strata, both between the excavations and below the maximum depth achieved by the investigation, these comments are for guidance only. It should also be noted that groundwater levels vary due to seasonal or other effects and may at times differ to those measured during the investigation. For investigations, which include environmental issues, the data obtained relate to the conditions which are relevant at the time of the investigation.

This report is not an engineering design and the figures and calculations enclosed to the report should be used by the Structural Engineer taking note that variations may apply depending on design loading, in techniques used, and in site conditions. Our recommendations should therefore not supersede the Engineer's design.

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expenses (including any consequential damages) resulting from the use of this Report or any information contained in this Report by a third party.

4. GROUND INVESTIGATION

The site work was carried out between 29th July and 7th August 2025, aimed to obtain geotechnical parameters in regards to the proposed development. The investigation and soil descriptions were carried out in general accordance with BS5930:2015 – Code of Practice for Site Investigations.

The ground investigation comprised the following works:

- 6No. boreholes advanced by drive-in sampler technique to a max 5.0m depth;
- 3No. rotary boreholes advanced by rotary rig to a max 15.3m depth;
- 3No. Ø35mm standpipes installed for future groundwater/gas monitoring;
- 6No. CBR tests using Dynamic Cone Penetration method
- 3No. soakaway tests following the procedure of the BRE 365;
- In situ testing and logging.

The borehole locations were determined approximately by reference to physical features present on the site at the time of investigation and the ground level at the borehole locations was not determined.

The locations of the exploratory holes are presented on **Figure 2**, the exploratory hole logs and the geotechnical testing results are presented in the **Appendix A** and **Appendix B**, respectively.

5. GEOTECHNICAL TESTING

The programme of geotechnical tests undertaken on samples obtained from the intrusive investigation is presented in **Table 1**, the main purpose of which was to classify the ground beneath the site and assist in determining its physical and mechanical properties. Where appropriate, testing was undertaken in accordance with BS 1377:1990 & EN ISO 14688-2 2004+A1:2013 Method of Tests for Soils for Civil Engineering Purposes and was carried out by the AVZGeoEng Ltd in house laboratory.

Tests carried out in order to classify the concrete class required on site have been undertaken following the procedures within BRE SD1:2005 by a UKAS accredited laboratory (Chemtech Environmental).

Table 1 – Summary of Geotechnical Testing Programme

Strata	Tests undertaken	No of Tests
Made Ground	pH & Water Soluble Sulphates	1
Weald Clay Formation - Mudstone	Natural Moisture Content	10
	Plasticity Index	10

Strata	Tests undertaken	No of Tests
	pH & Water Soluble Sulphates	9

6. GROUND CONDITIONS

The exploratory holes revealed that the site is underlain by a thin layer of Made Ground, over Weald Clay Formation – Mudstone, as bedrock geology. This appears to generally confirm the stratigraphical succession suggested by the published geological records.

The ground conditions are summarised in **Table 2** below.

Table 2 - General succession of strata encountered

Brief Description	Depth to top of stratum m.bgl	Thickness (m)
Made Ground	G.L.	0.20 to 0.7
Weald Clay Formation - Mudstone	0.2 to 0.7	Not proven

The Made Ground was found variable with limited thickness, consisting of sandy silt and clay or silty sand and gravel, with occasional to frequent roots, brick, coal ash, tarmac, mudstone and brick, concrete and glass fragments. More precise image of the made ground composition is presented in the borehole logs (**Appendix A**).

The Weald Clay Formation – Mudstone comprised firm to hard, slightly sandy clay or silty clay with occasional sandy laminas, with sublayers of very weak thinly laminated mudstone and siltstone. With depth the formation was turning into weathered thinly bedded extremely weak to weak siltstone and mudstone, with occasional thin limestone sublayers. The borehole logs (**Appendix A**) show a general image of the lithological composition of the encountered materials.

The measured and inferred soil parameters for the stratum are listed in **Table 3** below.

Table 3 – Summary of Soil Parameters for the Weald Clay Formation - Mudstone

Soil Parameters	Range	Results
Liquid Limit (%)	45 to 69	Appendix B
Plastic Limit (%)	22 to 28	Appendix B
Plastic Index (%)	23 to 46	Appendix B
Modified Plasticity Index (%)	23 to 46	Appendix B
Plasticity Term	Intermediate to high	
Volume Change Potential (NHBC)	Medium to high	
Moisture Content (%)	18.2 to 35.2	Appendix B
SPT 'N' Values	4 to >100	Appendix A, Figure 3

Soil Parameters	Range	Results
Undrained Shear Strength (kN/m ²) inferred by SPT ⁽¹⁾	17 to 429	Figure 4
Strength Term	Very soft to hard	

⁽¹⁾ SPT – Standard Penetration Tests

7. GROUNDWATER CONDITIONS

All boreholes remained dry during site works. Standpipes were installed in three of the boreholes for future groundwater or gas monitoring if required.

8. FOUNDATIONS

The ground conditions appear feasible for the design and construction of conventical spread foundations for the proposed development. Alternatively piled foundations could be adopted, particularly for any moderate to heavy or sensitive foundation loads.

The recommendations for the design and construction of spread and strip foundations in relation to ground conditions are set out in **Table 4**.

Table 4 – Design and construction of spread and strip foundations

Design/construction considerations	Design/construction recommendations						
Founding stratum	Weald Clay Formation - Mudstone						
Depth	Foundations should be taken to a minimum depth of 1.0 m below finished ground level and at least 0.1m into the founding stratum below any overlying made ground or to any greater depth required in respect of the special design considerations given below.						
Special design considerations	Due to the presence of shrinkable soils foundations should be designed taking into account all the normal precautions, including minimum depths, to minimise the risk of future foundation movements in accordance with NHBC Standards, or similar. The findings of the ground investigation indicate that foundations should be designed for shrinkable soils of high volume change potential.						
Bearing pressures for range of strip footings and square bases founded in the Weald Clay Formation	Net allowable bearing pressure kN/m ²						
		Strip Footings			Square bases		
	Width	0.5m	0.75m	1.0m	1m	1.25m	1.5m
	Depth						
	1.0m	113	114	114	133	133	133
	1.5m	148	149	149	174	174	174
	2.0m	174	175	176	205	205	205

Design/construction considerations	Design/construction recommendations
Basis of allowable bearing pressures	Each allowable bearing pressure includes an overall factor of safety of 3 against bearing capacity failure and with total settlements associated with the bearing pressure estimated to be less than 25mm.
Stability of excavations	Generally, the foundation excavations should remain stable in the short term. In the event that excavations are to remain open for longer periods, consideration should be given to the use of trench support systems.
Construction considerations	All foundation excavations should be inspected and any made ground, soft, organic or otherwise unsuitable materials removed and replaced with mass concrete. The proposed founding stratum is a relatively silt-rich soil, hence susceptible to rapid softening once exposed. Hence all foundation excavations should immediately be blinded with concrete or the full foundation constructed.

9. PILED FOUNDATIONS

The recommendations for the design and construction of piled foundations in relation to the ground conditions are set out in **Table 5**

Table 5 – Design and Construction of Piled Foundations

Design/construction considerations	Design/construction recommendations	
Pile type	The construction of bored or driven piles is considered technically feasible at this site.	
Possible constraints on choice of pile type	Driven piles may not be acceptable due to ground vibration and noise related problems.	
Temporary casing where groundwater is present	Bored piles may require temporary casing throughout their depth where presence of groundwater is possible. Use of CFA piles may overcome the issue.	
Man-made obstructions	The presence of buried sub-structures or other obstructions within made ground (expected some remains of the former structures,) may lead to some difficulty during piling, therefore it is recommended the pile contractor should take that in consideration when quoting the job. Where buried obstructions are encountered, it will be necessary to either relocate the pile(s) or make allowance for removing the obstruction.	
Hard strata	An allowance should be made for chiselling 'rock' bands (mudstone, siltstone or limestone) within the Weald Clay Formation.	
Soil and pile design parameters for Weald Clay	Adhesion Factor (α)	0.5/0.6
	Bearing Capacity Factor (N_c)	9

Design/construction considerations	Design/construction recommendations	
Formation - bored piles (cohesive soils)	Undrained Shear Strength (c_u)	70@1m + 15z kN/m ² where z = depth into clay
	Global Safety Factor	3.0/2.5
	Limiting Shaft Friction	140 kN/m ²
	Limiting Concrete Stress	7.5N/mm ²
Special Design Consideration	For piles in high volume change potential clay, the initial 3mbgl will need to be sleeved, or alternatively the design will need to allow for appropriate reinforcement for the full length of the pile able to withstand the tension forces that would occur due to the ground movements in the top stratum.	

10. RADON

Indicative Atlas of Radon in the UK indicates that the property is not in a Radon Affected Area, as less than 1% of properties are above the Action Level and no radon protective measures are necessary. However, it should be noted that radon need to be considered in relation to any development with a basement regardless of the location.

Additional information about Radon Affected Areas could be obtained from the publication 'Radon - Guidance on protective measures for new buildings BRE211 (BRE and HPA, 2007)' and the "Indicative Atlas of Radon in England and Wels" published by the Health Protection Agency and British Geological Survey, see www.ukradon.org for more details.

11. ROADS AND HARDSTANDING

In the initial meter below the proposed finished ground level the exploratory holes have revealed a soil profile comprising sandy silt and clay, silty sand and gravel or silty clay, with occasional sandy laminas.

In road/pavement design terms, the groundwater conditions are anticipated to comprise a low water table at least 1m below the pavement formation level.

The tests start from the current ground level and the results of in situ DCP testing are summarised in **Table 6**. The CBR values at and below the proposed formation level could be determined from the test diagrams presented in **Figure 5**.

Table 6 Summary of CBR values derived from in situ DCP tests

Test Location	Material Type	Minimum CBR value determined at or just below anticipated formation level
DCP1	Made Ground/Natural strata	3% (generally > 8%)
DCP2	Made Ground/Natural strata	4% (generally > 10%)

Test Location	Material Type	Minimum CBR value determined at or just below anticipated formation level
DCP3	Made Ground/Natural strata	3% (generally > 6%)
DCP4	Made Ground/Natural strata	6% (generally > 10%)
DCP5	Made Ground/Natural strata	3% (generally > 5%)
DCP6	Made Ground/Natural strata	2%

* Topsoil in a landscape area, hence starting values were ignored.

The cohesive sub-grade soils may not be susceptible to improvement by rolling with conventional compaction plant.

It should be noted that in-situ CBR readings can vary significantly depending on a number of factors, including the moisture content of the soil, and this can in turn vary depending on the weather conditions leading up to and during testing.

Therefore, the above results and recommendations should be used as preliminary for the design purposes and confirmed during the construction phase of the project.

12. SOAKAWAYS

Soakaways are used to store the immediate surface water run-off from hard surfaced areas, such as roofs or car parks, and allow for infiltration into the adjacent soil. They need to provide the necessary capacity to receive run-off from a subsequent storm. The time taken for discharge depends upon the soakaway shape and size, and the surrounding soil's infiltration characteristics. Soakaway may not be an appropriate solution in areas of ground that have low permeability and where the maximum seasonal water table may be at or above the base of the proposed soakaway.

3No. soakaway tests were carried out in trial pit at locations advised by the engineer as potentially suitable for the proposed soakaway, following the analogy and recommendations given in the BRE 365. For the purpose of the test, trial pits were excavated by machine and backfilled with arisings on completion.

Due to low infiltration rates, the water level in the trial pits failed to drain below the 25% level within the time available for the test, results are presented in the **Figure 6**.

The extremely low permeability recorded is considered to be a function of the typically high fines (clay and silt) contents of the sub-soils, hence the location is considered unsuitable for traditional shallow pit soakaways.

13. CHEMICAL ANALYSIS OF SOIL SAMPLES

The client requested preliminary contamination assessment by means of collecting limited number of samples and laboratory screening tests for presence of most common contaminants.

This report and scope of work carried out is intended to provide preliminary information for existence of contamination in the shallow ground around the site and this investigation was not designed to satisfy the

full requirements as defined by BS 10175:2001 “Code of Practice for the Investigation of Potentially Contaminated Sites”.

An asbestos survey of existing structures and infrastructure (as defined under Section 5(a) of the Control of Asbestos Regulations 2012) was beyond the brief of this report. The risk assessment has been undertaken on the basis that should asbestos be identified within buildings or infrastructure, these materials will be removed appropriately by licensed contractors and asbestos materials disposed of in accordance with legal requirements prior to demolition or other works in order to avoid contaminating soils at the site.

Visual/olfactory evidence of contamination was not encountered during the site work into any of the trial pits or boreholes.

The programme of chemical tests was undertaken on samples obtained from the intrusive investigation as presented in **Table 7**. Testing was undertaken by a UKAS accredited laboratory (Chemtech Environmental), MCERTS accredited test methods were specified where applicable.

Table 7 – Summary of Chemical Testing Programme

Test Suite	Number of Samples	Soil Tested
Metals Suite - As, Ba, Be, WSB, Cd, Cr, Cu, Pb, Hg, Ni, Se, V & Zn	5	Made Ground
Asbestos Screening	5	
TPH CWG with BTEX (Soil)	5	
Speciated PAH EPA16 by GC-MS	5	
Total organic carbon	5	

Chemical analyses have been performed on a total of 5No. soil samples, from the made ground, down to a maximum depth of 0.6m below existing ground level. In addition to the chemical analyses, a total of 5No. samples were screened in the laboratory for the presence of asbestos fibres.

The visual inspection at the laboratory identified no materials suspected of potentially containing asbestos and the scheduled laboratory screening for asbestos found no detectable asbestos fibres within the sample.

The full chemical results are presented within **Appendix B**. The results have been assessed with respect to Human Health Risk Assessment conducted on the results of the laboratory tests on soils sampled from the site were evaluated by the Atkins ATRISK^{soil} SSVs derived using CLEA v1.071 software and up to date exposure assumptions based on the principles of minimal toxicological risk at 1% and 6% soil organic matter. Given the proposed end use, the site was treated as commercial.

In conclusion, the findings of this limited investigation and testing have provided no evidence of significant ground contamination on site with regards to its proposed end-use. It is recommended that imported topsoil/subsoil to be used in any landscaped areas should be clean and suitably tested.

Adequate health and safety measures may be required during groundworks. In respect of the risk posed to the construction workers who will come into contact with the made ground during the development works, protection measures including regular use of standard PPE equipment (gloves, hard hats, safety boots etc.) and maintenance of self-hygiene should be sufficient to overcome this issue.

Water supply infrastructure should be considered the most sensitive receptor with respect to buried services. It is recommended that the results of this investigation be presented to the water utility company as soon as reasonably practicable in order to confirm the pipe material. As a matter of good practice, and to maximize the protection to utilities, it is recommended that clean, granular backfill is used in service runs and that marker tapes are used for all buried services.

It is possible however, that groundwork may encounter different conditions from those revealed by the site investigation. It is therefore recommended that the groundworks be monitored for previously undetected suspect materials and if found appropriate additional testing and advice is sought.

It is recommended that this report is referenced in the site Health and Safety Plan and that normal good hygiene practice is observed during the works and subsequent building and grounds maintenance

14. WASTE CLASSIFICATION OF SOIL SAMPLES

The client requested Waste Acceptance Criteria (WAC) assessment of the site by means of collecting limited number of samples and laboratory tests.

Chemical WAC test analyses have been performed on single soil sample from the made ground & natural soil down to a maximum depth of 1.0m below existing ground level.

Testing was undertaken by a UKAS accredited laboratory (Chemtech Environmental), MCERTS accredited test methods were specified where applicable. The full WAC test results are presented within **Appendix C**. All soil samples scheduled for laboratory testing are also inspected visually on receipt at the laboratory for the presence of materials potentially containing asbestos, e.g. fragments of asbestos-cement products.

The Laboratory WAC tests indicated all results were found to be below the threshold for inert and is likely therefore to be classified as Inert waste, and as such may either be disposed of to a suitable landfill site or re-use on another project where inert fill material is required.

It is important to note that the assessment given in this report is for guidance only and it is always necessary to confirm the actual classification with prospective landfill operators prior to disposal.

15. CHEMICAL ATTACK ON BURIED CONCRETE

The results of chemical tests carried out on soil samples from the made ground and natural soil indicate 2:1 water soil extract sulphate with max content of 2040 mg/l with generally alkaline pH values.

These results indicate that, in accordance with BRE Special Digest 1:2005 Concrete in aggressive ground, the Aggressive Chemical Environment for Concrete (ACEC) Classification is **AC-2** with a Design Sulphate Class for the site of **DS-3**.

This assumes nominally static groundwater conditions and that no significantly disturbed clay comes into contact with concrete foundations or structures. This would apply for designing the concrete class for shallow foundations and concrete elements.

16. RECOMMENDATION IN RELATION TO FOUNDATIONS

From the results of site investigation, the following recommendations are given in respect to ground engineering and foundation design of the underlying soils:

- The site investigation consisted of 6 (six) drive-in sampler boreholes up to 5.0mbgl and 3 (three) rotary boreholes up to 15.3mbgl, aimed to confirm the ground profile below the site and recover samples for laboratory testing. The boreholes remained dry during site works.
- The investigation found that site is underlain by a thin layer of Made Ground, over Weald Clay Formation - Mudstone as bedrock geology. From geotechnical point of view the investigation revealed ground conditions appear suitable for the design and construction of conventional spread foundations for the proposed development subject to appropriate design. Alternatively piled foundations could be adopted, particularly for any moderate to heavy or sensitive foundation loads.
- Allowable bearing Capacity or pressure is a preliminary estimate of the ultimate bearing resistance of the ground, selected on the basis of the soil description and in situ tests, in addition taking a presumed degree of settlement into account. It has traditionally been used to check simple foundation designs. The use of this method is acceptable under BS EN 1997-116 as prescriptive method with allowable bearing pressures termed 'presumed bearing resistance'.
- Foundations should be taken to a minimum depth of 1.0m below finished ground level and at least 0.1m into the founding stratum below any overlying made ground or to any greater depth required in respect of the special design considerations given below. Depends on the depth and shape of the proposed footings, spread foundations should be designed using a net presumed bearings resistance of between 113kN/m² to 205kN/m², as advised in the **Table 4** of this Report.
- The presumed bearing resistance depends on the proposed depth of foundations, width of the footings and its tolerance to settlements. The preliminary presumed bearing resistance assuming a stated depth and width of foundation and assumes that settlements of up to 25mm can be tolerated by the structure. For alternative foundation depths and dimensions of the footings, or if the proposed structure has different short or long term movement requirements then the above assessment should be re-evaluated.
- Special Design Consideration:
 1. Due to the presence of shrinkable soils foundations should be designed taking into account all the normal precautions, including minimum depths, to minimise the risk of future foundation movements in accordance with NHBC Standards, or similar. The findings of the ground investigation indicate that foundations should be designed for shrinkable soils of high volume change potential.
- Advise on design and construction of piled foundations is included in **Table 4** of this Report, which includes advice for pile design based on SPT tests results in soil. In rock piles could also be designed using rock socket method. It should be stressed that these parameters are suitable to be

used for preliminary design purposes, the detailed design should be provided by the specialist piling contractor and would depend of the type of piles adopted for the project.

- The water-soluble soil extract sulphate contents tests indicate that, in accordance with BRE Special Digest 1:2005 Concrete in aggressive ground, the Aggressive Chemical Environment for Concrete (ACEC) Classification is **AC-2** with a Design Sulphate Class for the site of **DS-3**.
- All foundation excavations should be taken to depth reaching the competent foundation stratum, inspected and approved by a competent person before casting the concrete. Any made ground, soft, organic or otherwise unsuitable materials removed and replaced with mass concrete.

We trust that the above meets with your requirements, however, if should wish to discuss any of the matters further, please do not hesitate to contact us.

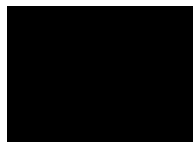
Yours Sincerely,

For AVZGeoEng Ltd

Author:

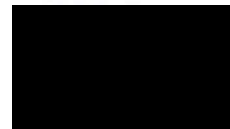
BSc (Hons) Silvanka Kadieva-Zarkovska

Senior Geotechnical Engineer

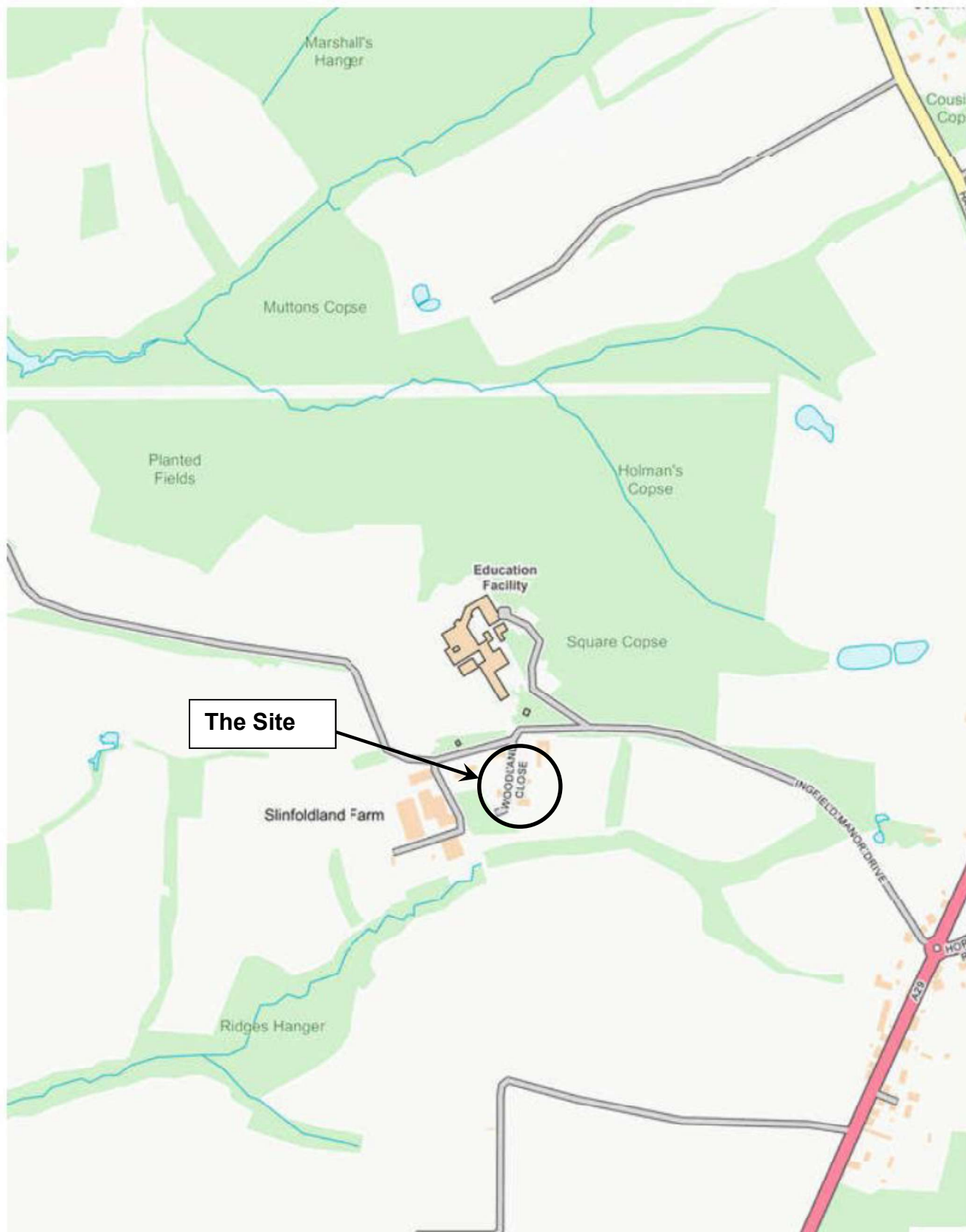


Approved:

Dipl. Ing. Boban Zarkovski CEng MICE



FIGURES



Client: Salutem LD Bidco IV Ltd

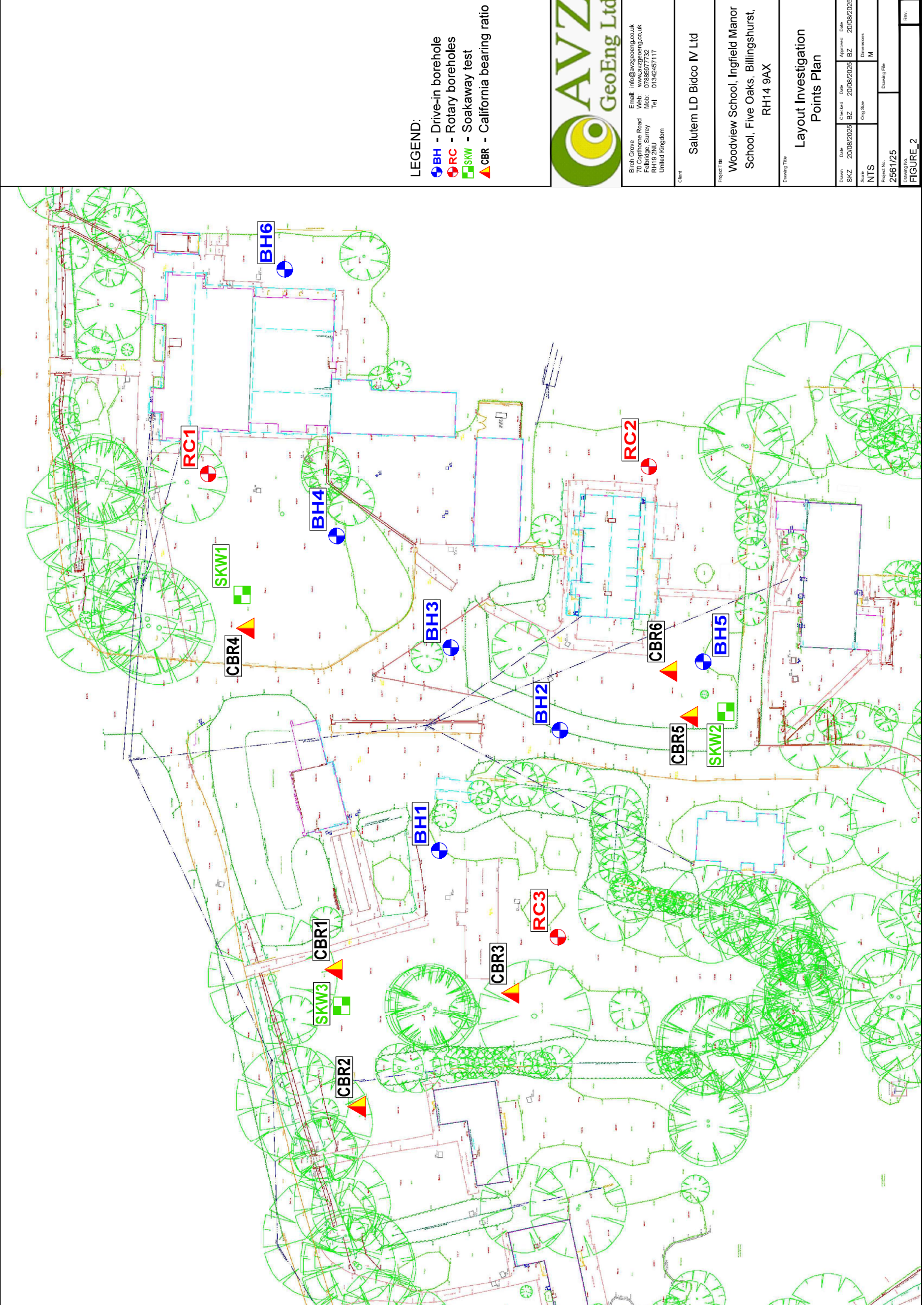
Scale: NTS

Site: Woodview School, Ingfield Manor School, Five Oaks, Billingshurst, RH14 9AX

SITE LOCATION PLAN

Job No: 2561/25

Figure No: 1



LEGEND:

- BH - Drive-in borehole
- ⊕ RC - Rotary boreholes
- ⊕ SKW - Soakaway test
- ▲ CBR - California bearing ratio



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Client: Saltem LD Bidco IV Ltd

Project File:
Woodview School, Ingfield Manor
School, Five Oaks, Billingshurst,
RH14 9AX

Drawing Title:
Layout Investigation
Points Plan

Drawn	Date	Checked	Date	Approved	Date
SKZ	20/08/2025	BZ	20/08/2025	BZ	20/08/2025
NTS		Orig Size		Dimensions	
				M	
Project No.	2561725				
Drawing No.					
Rev.					

	Site	SPT N60 Vs Depth Profile	
	Woodview School, Ingfield Manor School, Five Oaks, Billingshurst, RH14 9AX	Client:	Job Number
		Salutem LD Bidco IV Ltd	2561/25

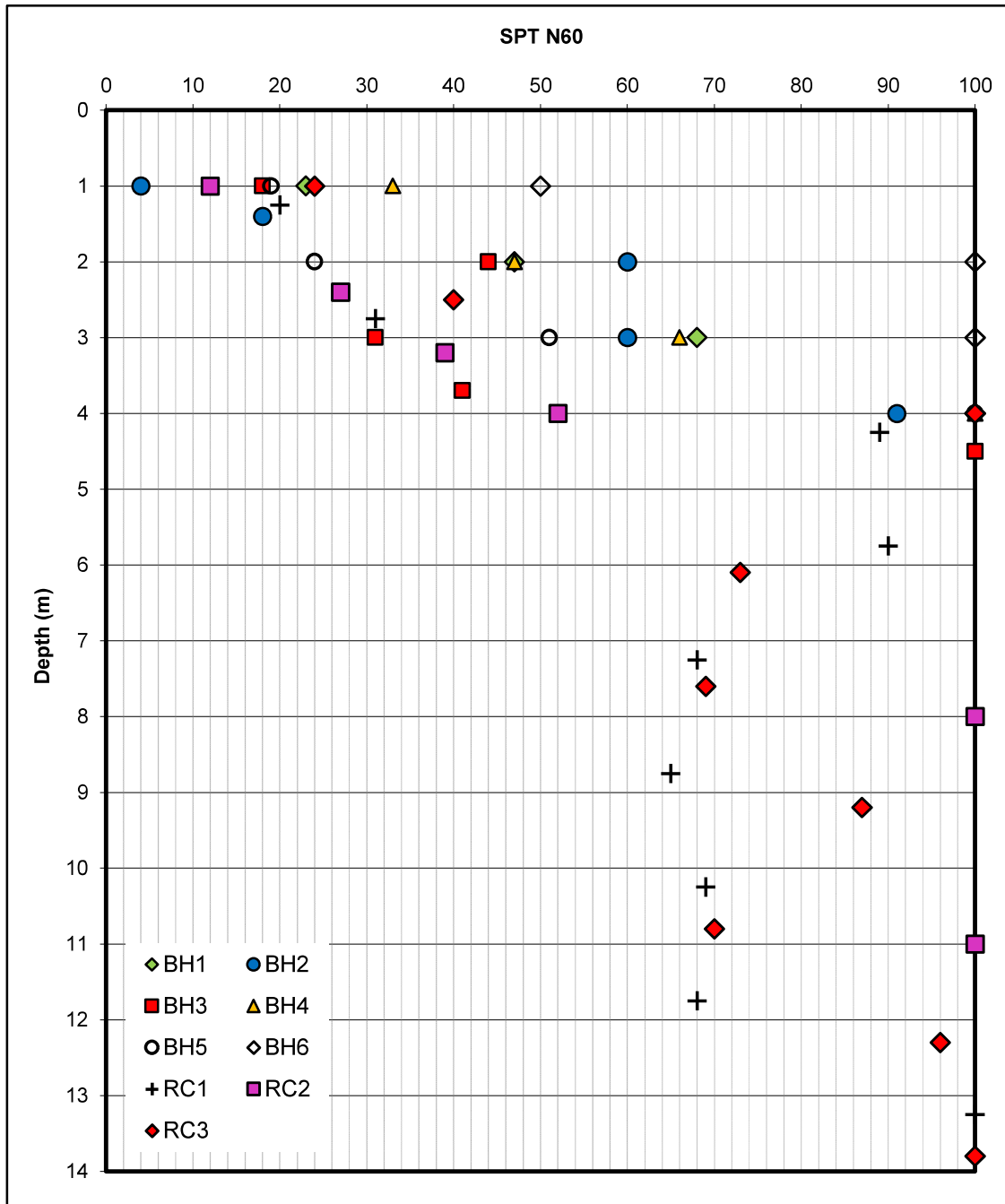


Figure 3

	Site	c_u Vs Depth Profile	
	Woodview School, Ingfield Manor School, Five Oaks, Billingshurst, RH14 9AX	Client: Salutem LD Bidco IV Ltd	Job Number 2561/25

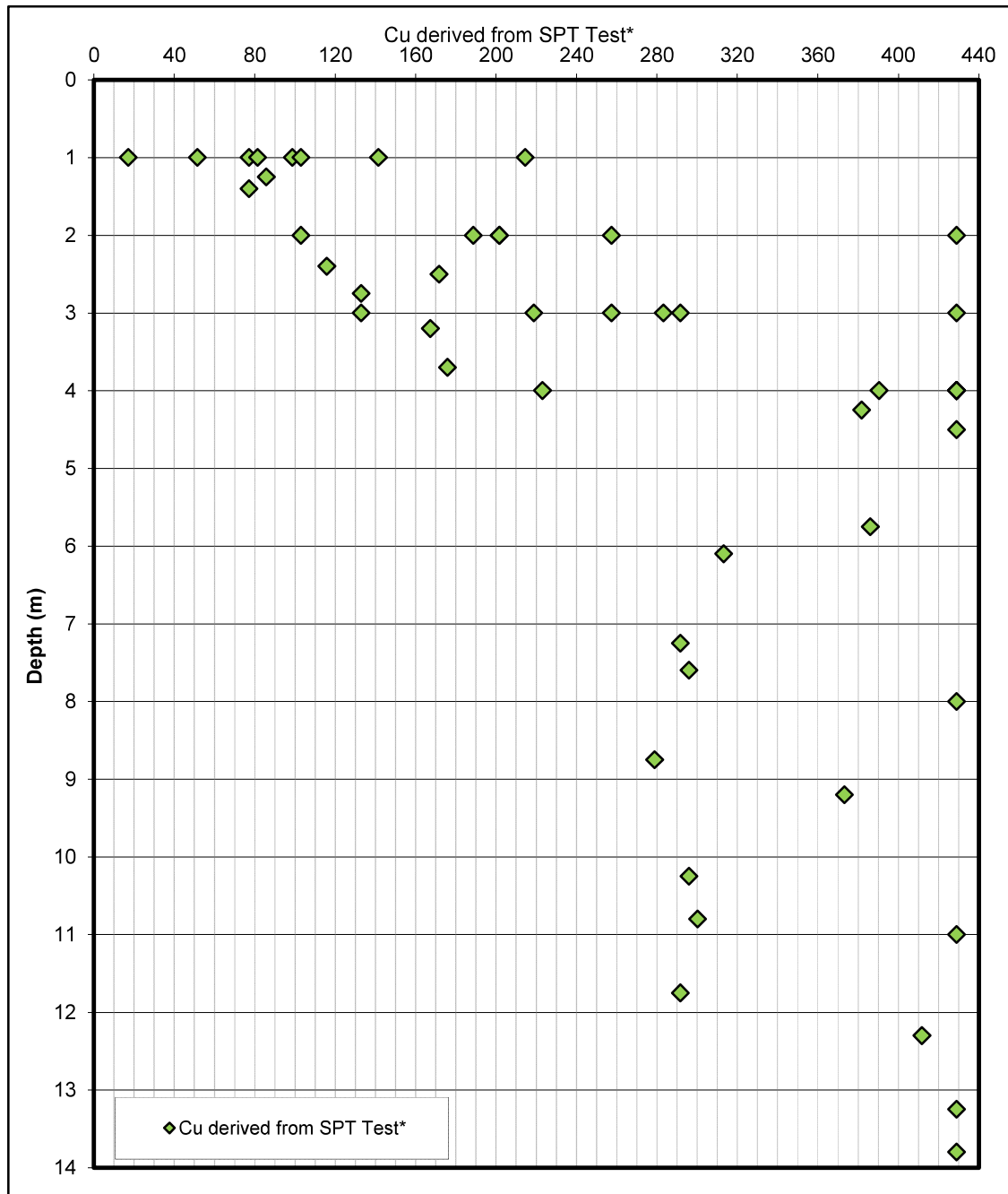


Figure 4



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IN SITU DCP TEST

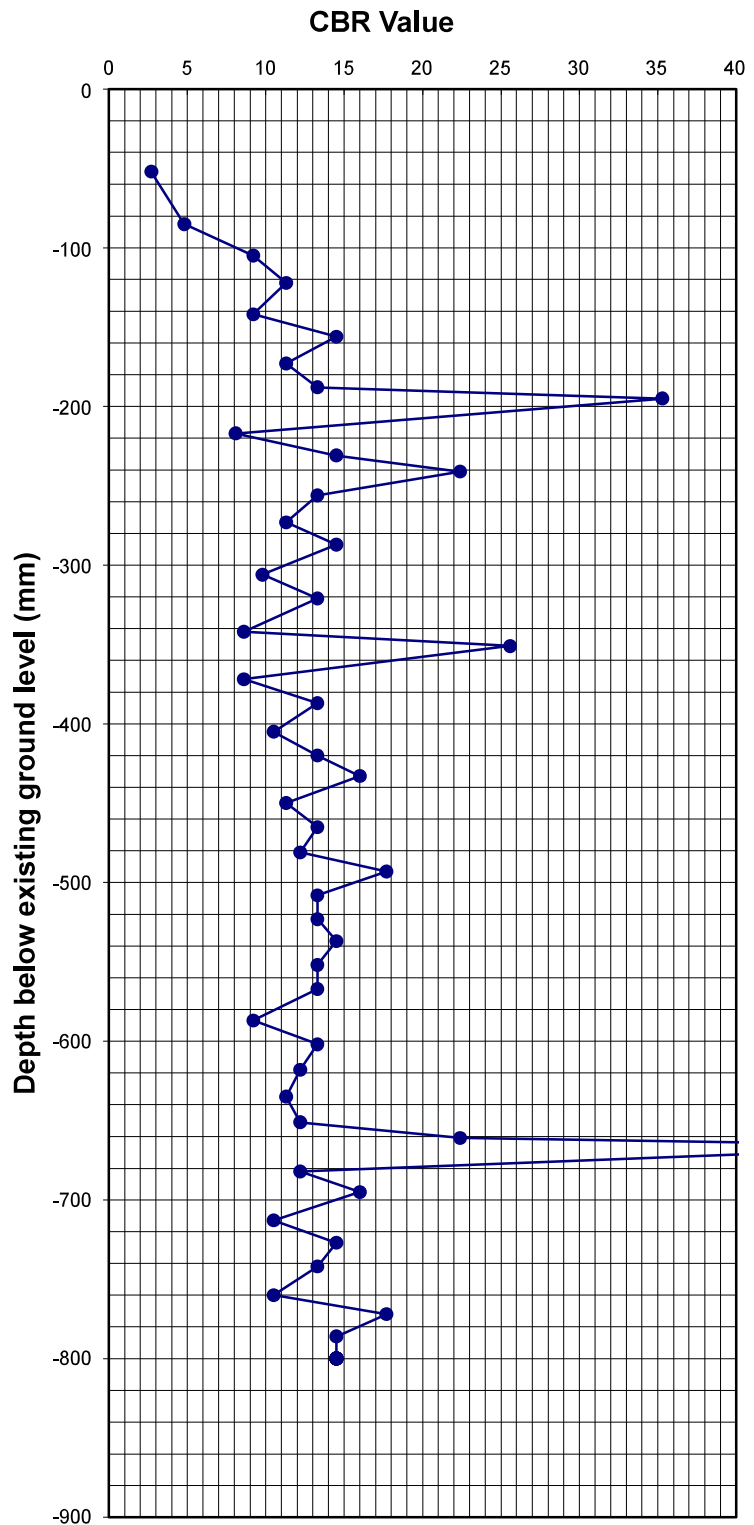
DCP1

Site:
Woodview School, Ingfield Manor School, Five Oaks,
Billingshurst, RH14 9AX

Client:
Salutem LD Bidco IV Ltd

Depth (mm)
GL

Number of blows	Reading (mm)	Pen/ blow (mm)	Depth (mm)	CBR
0	110	0	0	
1	162	52	-52	3
2	195	33	-85	5
3	215	20	-105	9
4	232	17	-122	11
5	252	20	-142	9
6	266	14	-156	15
7	283	17	-173	11
8	298	15	-188	13
9	305	7	-195	35
10	327	22	-217	8
11	341	14	-231	15
12	351	10	-241	22
13	366	15	-256	13
14	383	17	-273	11
15	397	14	-287	15
16	416	19	-306	10
17	431	15	-321	13
18	452	21	-342	9
19	461	9	-351	26
20	482	21	-372	9
21	497	15	-387	13
22	515	18	-405	11
23	530	15	-420	13
24	543	13	-433	16
25	560	17	-450	11
26	575	15	-465	13
27	591	16	-481	12
28	603	12	-493	18
29	618	15	-508	13
30	633	15	-523	13
31	647	14	-537	15
32	662	15	-552	13
33	677	15	-567	13
34	697	20	-587	9
35	712	15	-602	13
36	728	16	-618	12
37	745	17	-635	11
38	761	16	-651	12
39	771	10	-661	22
40	776	5	-666	54
41	792	16	-682	12
42	805	13	-695	16
43	823	18	-713	11
44	837	14	-727	15
45	852	15	-742	13
46	870	18	-760	11
47	882	12	-772	18
48	896	14	-786	15
49	910	14	-800	15



Comments: DCP Test Started from GL

Job Number: 2561/25

Figure: 5_1



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01342 457117; 07885977732

IN SITU DCP TEST

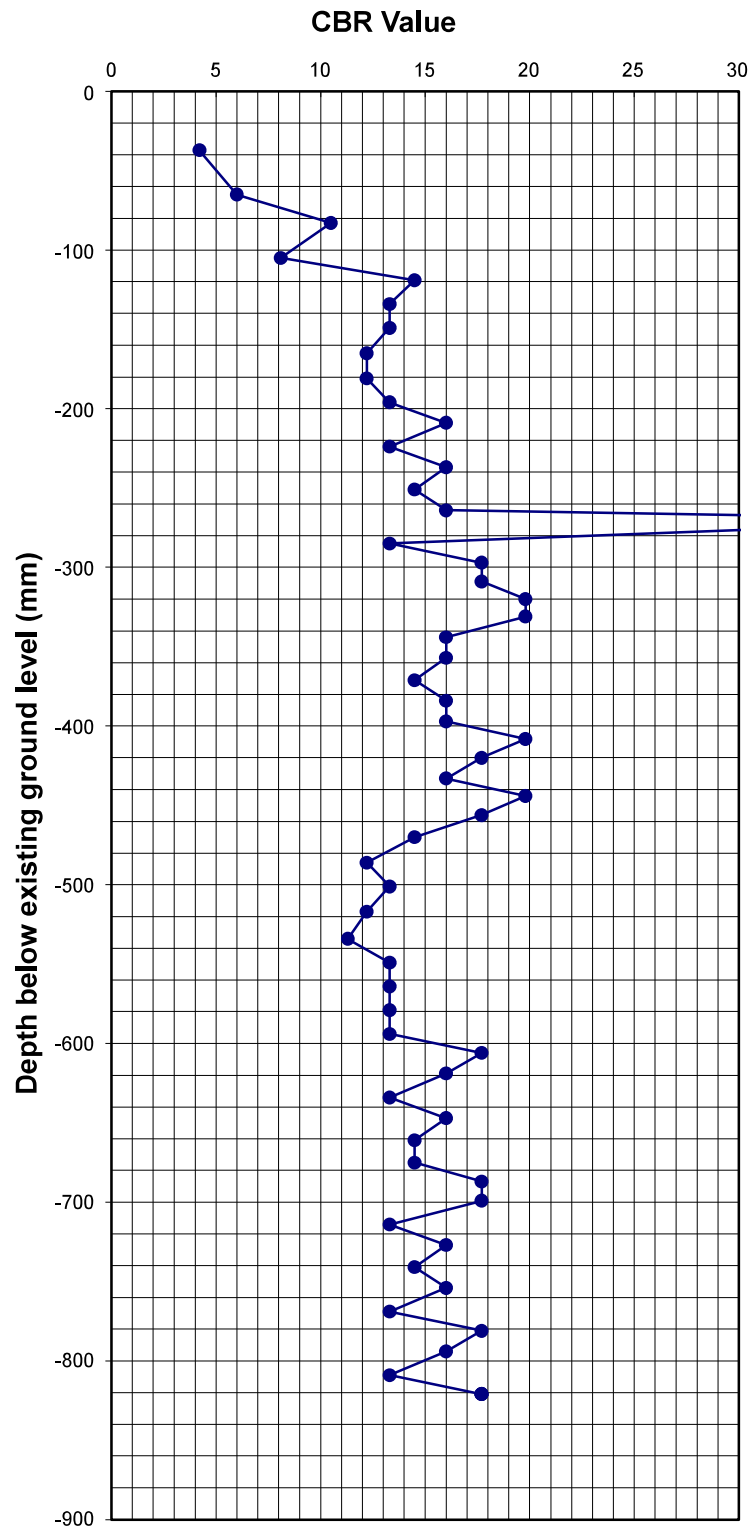
DCP2

Site:
Woodview School, Ingfield Manor School, Five Oaks,
Billingshurst, RH14 9AX

Client:
Salutem LD Bidco IV Ltd

Depth (mm)
GL

Number of blows	Reading (mm)	Pen/ blow (mm)	Depth (mm)	CBR
0	111	0	0	
1	148	37	-37	4
2	176	28	-65	6
3	194	18	-83	11
4	216	22	-105	8
5	230	14	-119	15
6	245	15	-134	13
7	260	15	-149	13
8	276	16	-165	12
9	292	16	-181	12
10	307	15	-196	13
11	320	13	-209	16
12	335	15	-224	13
13	348	13	-237	16
14	362	14	-251	15
15	375	13	-264	16
16	381	6	-270	43
17	396	15	-285	13
18	408	12	-297	18
19	420	12	-309	18
20	431	11	-320	20
21	442	11	-331	20
22	455	13	-344	16
23	468	13	-357	16
24	482	14	-371	15
25	495	13	-384	16
26	508	13	-397	16
27	519	11	-408	20
28	531	12	-420	18
29	544	13	-433	16
30	555	11	-444	20
31	567	12	-456	18
32	581	14	-470	15
33	597	16	-486	12
34	612	15	-501	13
35	628	16	-517	12
36	645	17	-534	11
37	660	15	-549	13
38	675	15	-564	13
39	690	15	-579	13
40	705	15	-594	13
41	717	12	-606	18
42	730	13	-619	16
43	745	15	-634	13
44	758	13	-647	16
45	772	14	-661	15
46	786	14	-675	15
47	798	12	-687	18
48	810	12	-699	18
49	825	15	-714	13
50	838	13	-727	16
51	852	14	-741	15
52	865	13	-754	16
53	880	15	-769	13
54	892	12	-781	18
55	905	13	-794	16
56	920	15	-809	13
57	932	12	-821	18



Comments: DCP Test Started from GL

Job Number: 2561/25

Figure: 5_2



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IN SITU DCP TEST

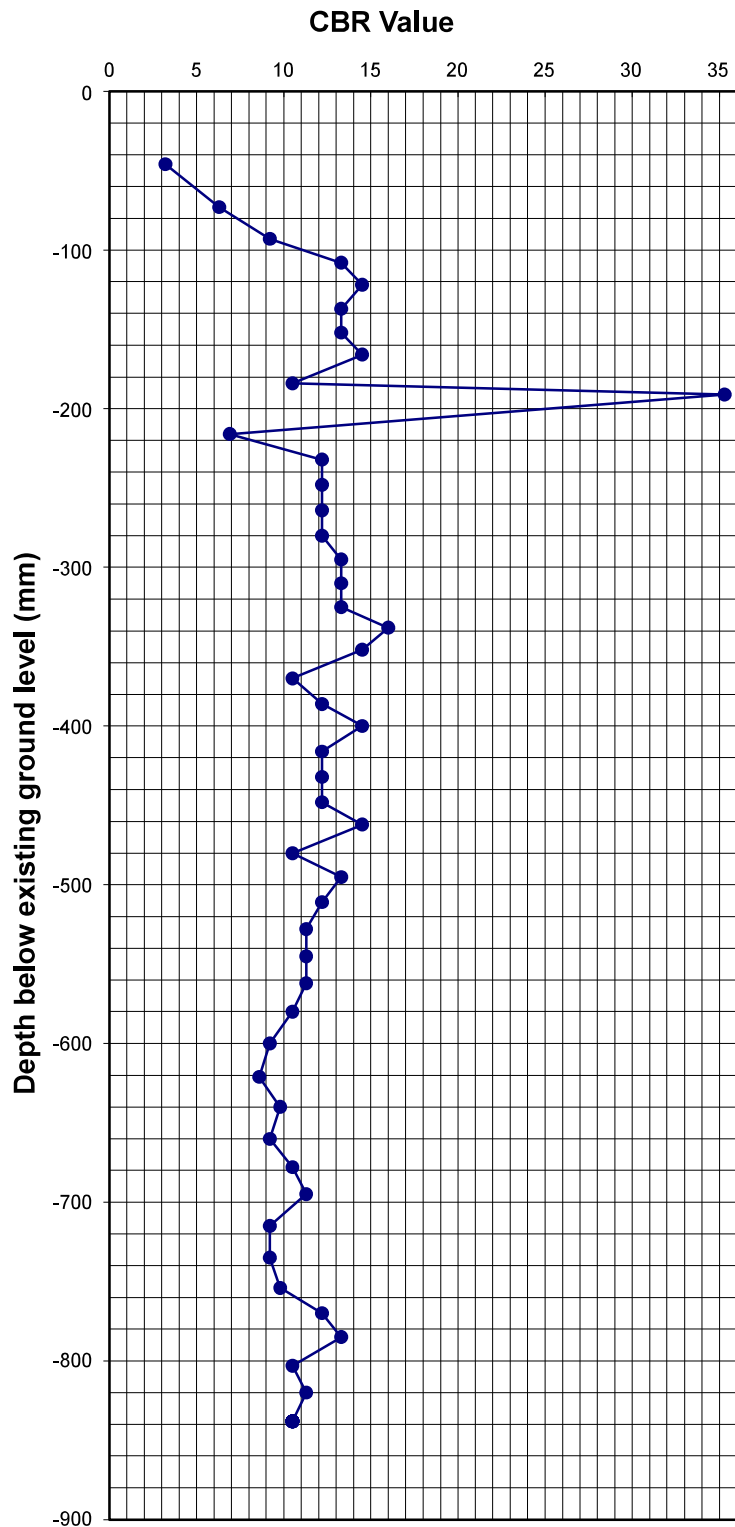
DCP3

Site:
Woodview School, Ingfield Manor School, Five Oaks,
Billingshurst, RH14 9AX

Client:
Salutem LD Bidco IV Ltd

Depth (mm)
GL

Number of blows	Reading (mm)	Pen/ blow (mm)	Depth (mm)	CBR
0	110	0	0	
1	156	46	-46	3
2	183	27	-73	6
3	203	20	-93	9
4	218	15	-108	13
5	232	14	-122	15
6	247	15	-137	13
7	262	15	-152	13
8	276	14	-166	15
9	294	18	-184	11
10	301	7	-191	35
11	326	25	-216	7
12	342	16	-232	12
13	358	16	-248	12
14	374	16	-264	12
15	390	16	-280	12
16	405	15	-295	13
17	420	15	-310	13
18	435	15	-325	13
19	448	13	-338	16
20	462	14	-352	15
21	480	18	-370	11
22	496	16	-386	12
23	510	14	-400	15
24	526	16	-416	12
25	542	16	-432	12
26	558	16	-448	12
27	572	14	-462	15
28	590	18	-480	11
29	605	15	-495	13
30	621	16	-511	12
31	638	17	-528	11
32	655	17	-545	11
33	672	17	-562	11
34	690	18	-580	11
35	710	20	-600	9
36	731	21	-621	9
37	750	19	-640	10
38	770	20	-660	9
39	788	18	-678	11
40	805	17	-695	11
41	825	20	-715	9
42	845	20	-735	9
43	864	19	-754	10
44	880	16	-770	12
45	895	15	-785	13
46	913	18	-803	11
47	930	17	-820	11
48	948	18	-838	11



Comments: DCP Test Started from GL

Job Number: 2561/25

Figure: 5_3



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IN SITU DCP TEST

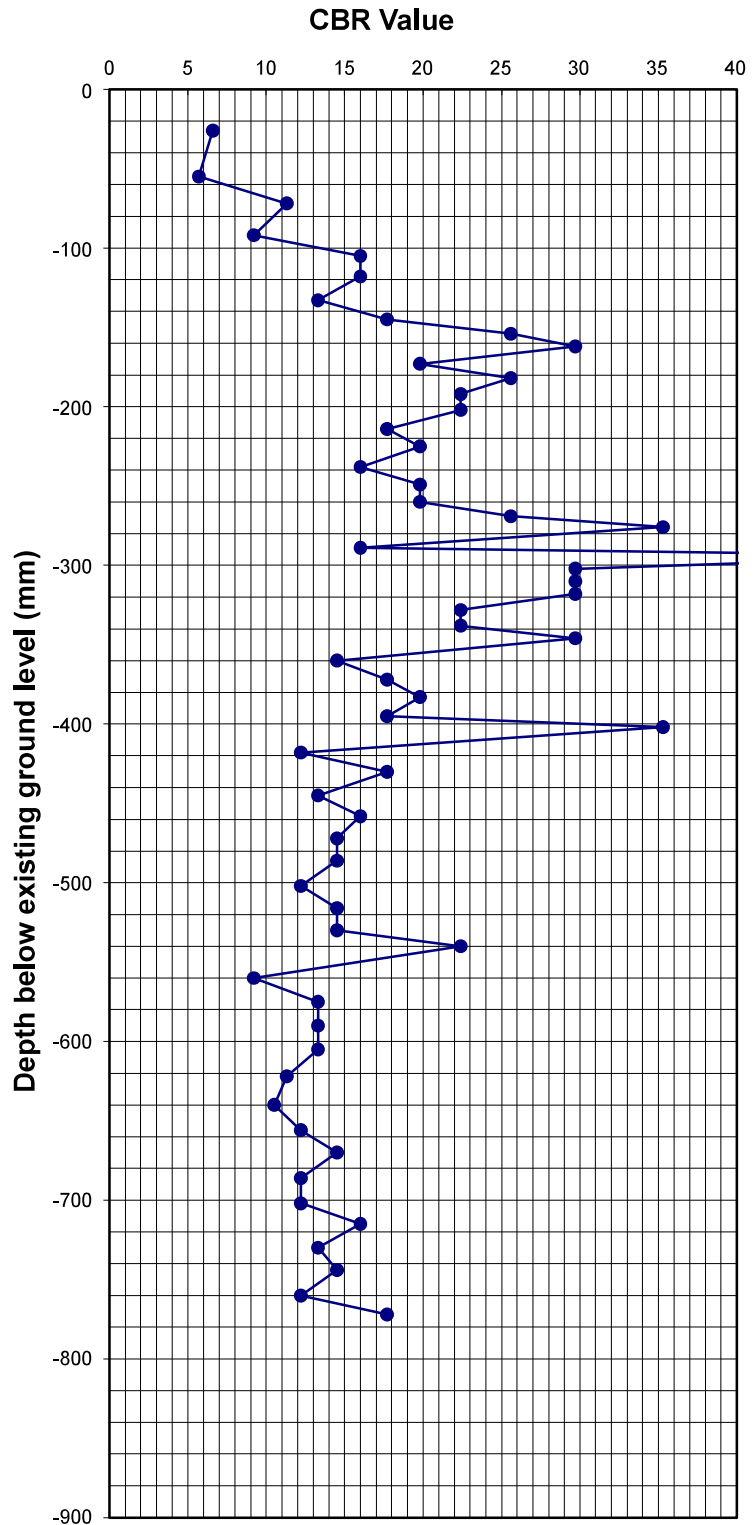
DCP4

Site:
Woodview School, Ingfield Manor School, Five Oaks,
Billingshurst, RH14 9AX

Client:
Salutem LD Bidco IV Ltd

Depth (mm)
GL

Number of blows	Reading (mm)	Pen/ blow (mm)	Depth (mm)	CBR
0	110	0	0	
1	136	26	-26	7
2	165	29	-55	6
3	182	17	-72	11
4	202	20	-92	9
5	215	13	-105	16
6	228	13	-118	16
7	243	15	-133	13
8	255	12	-145	18
9	264	9	-154	26
10	272	8	-162	30
11	283	11	-173	20
12	292	9	-182	26
13	302	10	-192	22
14	312	10	-202	22
15	324	12	-214	18
16	335	11	-225	20
17	348	13	-238	16
18	359	11	-249	20
19	370	11	-260	20
20	379	9	-269	26
21	386	7	-276	35
22	399	13	-289	16
23	404	5	-294	54
24	412	8	-302	30
25	420	8	-310	30
26	428	8	-318	30
27	438	10	-328	22
28	448	10	-338	22
29	456	8	-346	30
30	470	14	-360	15
31	482	12	-372	18
32	493	11	-383	20
33	505	12	-395	18
34	512	7	-402	35
35	528	16	-418	12
36	540	12	-430	18
37	555	15	-445	13
38	568	13	-458	16
39	582	14	-472	15
40	596	14	-486	15
41	612	16	-502	12
42	626	14	-516	15
43	640	14	-530	15
44	650	10	-540	22
45	670	20	-560	9
46	685	15	-575	13
47	700	15	-590	13
48	715	15	-605	13
49	732	17	-622	11
50	750	18	-640	11
51	766	16	-656	12
52	780	14	-670	15
53	796	16	-686	12
54	812	16	-702	12
55	825	13	-715	16
56	840	15	-730	13
57	854	14	-744	15
58	870	16	-760	12
59	882	12	-772	18



Comments: DCP Test Started from GL

Job Number: 2561/25

Figure: 5_4



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IN SITU DCP TEST

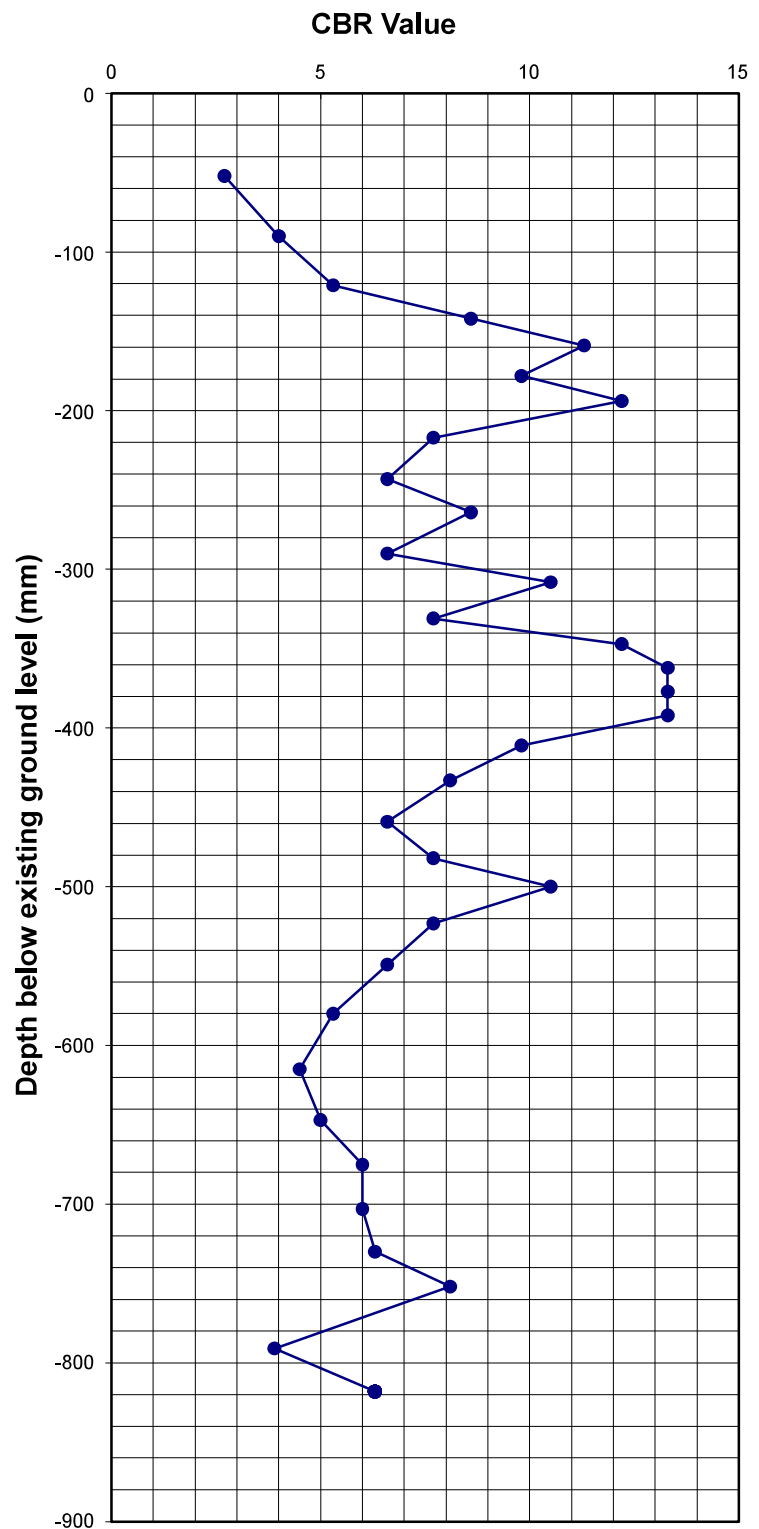
DCP5

Site:
Woodview School, Ingfield Manor School, Five Oaks,
Billingshurst, RH14 9AX

Client:
Salutem LD Bidco IV Ltd

Depth (mm)
GL

Number of blows	Reading (mm)	Pen/ blow (mm)	Depth (mm)	CBR
0	150	0	0	
1	202	52	-52	3
2	240	38	-90	4
3	271	31	-121	5
4	292	21	-142	9
5	309	17	-159	11
6	328	19	-178	10
7	344	16	-194	12
8	367	23	-217	8
9	393	26	-243	7
10	414	21	-264	9
11	440	26	-290	7
12	458	18	-308	11
13	481	23	-331	8
14	497	16	-347	12
15	512	15	-362	13
16	527	15	-377	13
17	542	15	-392	13
18	561	19	-411	10
19	583	22	-433	8
20	609	26	-459	7
21	632	23	-482	8
22	650	18	-500	11
23	673	23	-523	8
24	699	26	-549	7
25	730	31	-580	5
26	765	35	-615	5
27	797	32	-647	5
28	825	28	-675	6
29	853	28	-703	6
30	880	27	-730	6
31	902	22	-752	8
32	941	39	-791	4
33	968	27	-818	6



Comments: DCP Test Started from GL

Job Number: 2561/25

Figure: 5_5



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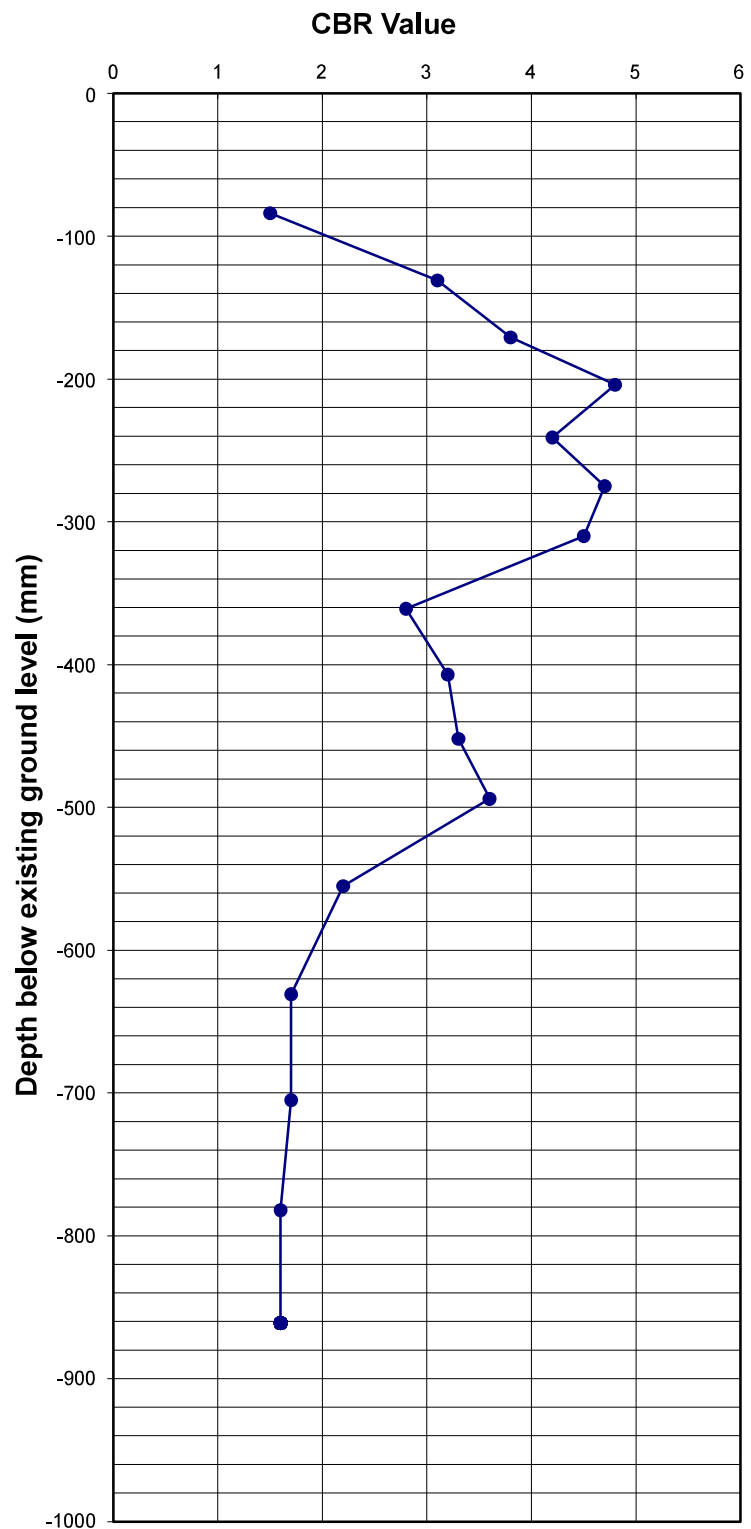
DCP6

Site:
Woodview School, Ingfield Manor School, Five Oaks,
Billingshurst, RH14 9AX

Client:
Salutem LD Bidco IV Ltd

Depth (mm)
GL

Number of blows	Reading (mm)	Pen/ blow (mm)	Depth (mm)	CBR
0	129	0	0	
1	213	84	-84	2
2	260	47	-131	3
3	300	40	-171	4
4	333	33	-204	5
5	370	37	-241	4
6	404	34	-275	5
7	439	35	-310	5
8	490	51	-361	3
9	536	46	-407	3
10	581	45	-452	3
11	623	42	-494	4
12	684	61	-555	2
13	760	76	-631	2
14	834	74	-705	2
15	911	77	-782	2
16	990	79	-861	2



Comments: DCP Test Started from GL

Job Number: 2561/25

Figure: 5_6

