

### 3. Recommendations

The table below shows the results from the modelling that was carried out. The results show that the system has sufficient capacity to support the new demand of 37.73 l/s l/s.

| DMA  | POC Address and Model Node   | Demand Scenario  | Pressure Available at the POC (m) |                                |
|------|--|--|-----------------------------------|--------------------------------|
|      |  |  | Model Pressure                    | Telemetry Adjustment (-3.00 m) |
| BR12 | 200mm PVC main in in<br>Peveril Road, Crawley<br>RH11 0TH<br><br>Grid Ref.<br>X: 524139<br>Y: 136465<br><br>Node – 6002538 | Before Additional Demand   | 34.69                             | 31.69                          |
|      |  | After Additional Demand (37.73 l/s)                              | 33.26                             | 30.26                          |
|      |  | Before Additional Demand with Fire Demand (20.00 l/s)            | 34.05                             | 31.05                          |
|      |  | After Additional Demand with Fire Demand (37.73 l/s + 20.00 l/s) | 31.77                             | 28.77                          |

## Appendix 5 Surface Water connection points

**SURFACE WATER DRAINAGE STRATEGY:**

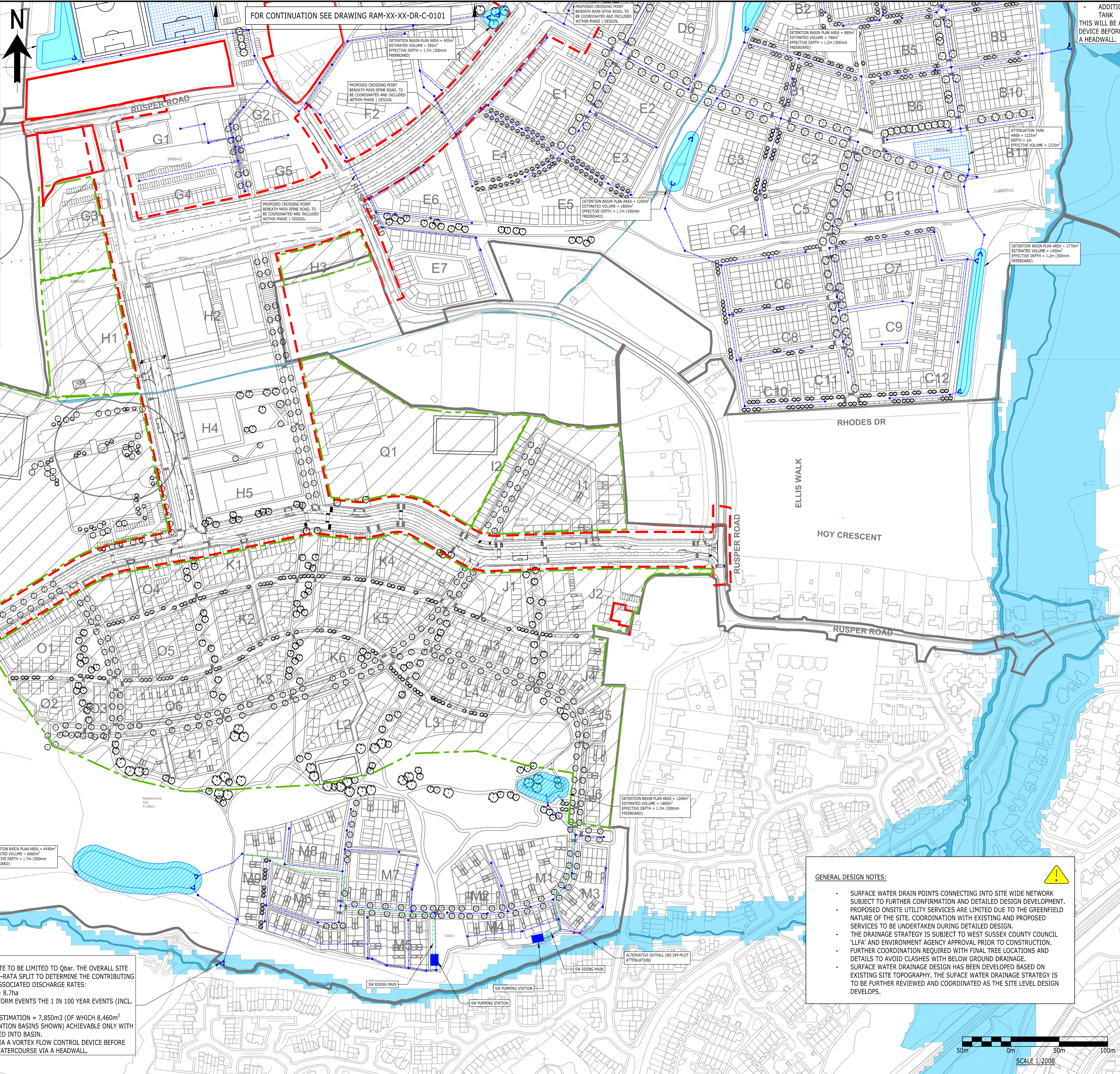
THE SITE BOUNDARY ACCOUNTS FOR 203ha HOWEVER, OF THE EXISTING SITE, IT IS CONSIDERED THAT THE DEVELOPED AREA CONTRIBUTING TO CHANGES IN THE DRAINING CHARACTERISTICS OF THE EXISTING GREENFIELD SITE ACCOUNTS FOR A TOTAL AREA OF 107ha. EXISTING WATERCOURSE DETAILS, LOCATION AND LEVELS TO BE CONFIRMED BY FURTHER SITE INVESTIGATIONS TO AID WITH DESIGN FOR DISCHARGE CONNECTIONS. NO DITCHES OR WATERCOURSES HAVE BEEN IDENTIFIED AS REQUIRING DIVERSION AS PART OF THE DEVELOPMENT PROPOSALS.

THE PROPOSED SITE WIDE DRAINAGE STRATEGY WILL COMPLY WITH THE BELOW INCLUSIONS FOR ONSITE SURFACE WATER MANAGEMENT AND MITIGATE FLOOD RISK OFF SITE:

- PROPOSED DISCHARGE RATE IS TO RETAIN GREENFIELD RUNOFF RATES TO BE RESTRICTED TO  $Q_{bar} = 300.38 \text{ L/S}$
- INITIAL STORAGE VOLUME REQUIRED FOR THE MAIN SITE TO MANAGE STORM EVENTS UP TO AND INCLUDING THE 1:100 YEARS STORM EVENT WITH THE INCLUSION FOR 40% CLIMATE CHANGE IS ESTIMATED TO BE  $= 52,113\text{m}^3$
- ADD: THE TOTAL STORAGE ACCOMMODATED AS PART OF THE SITE WIDE STRATEGY THROUGH DETENTION BASINS/ BELOW GROUND STORAGE TANKS IS  $= 36,985\text{m}^3$

PROPOSED SURFACE WATER DRAINAGE STRATEGY IS BASED ON DISCHARGE RATE OF RESTRICTING EXISTING GREEN FIELD RUNOFF TO  $Q_{bar}$  FOR THE PROPOSED DEVELOPMENT THROUGH THE USE OF ATTENUATION. IT IS NOT ANTICIPATED THAT THE STORAGE VOLUME IDENTIFIED ABOVE WOULD BE SUFFICIENT ON ITS OWN TO BE MANAGED WITHIN THE OVERALL SITE WIDE STORAGE AS SHOWN. IT WILL THEREFORE BE NECESSARY FOR THE REMAINING ATTENUATION TO BE PROVIDED WITHIN THE DEVELOPMENT PLOTS VIA SUSTAINABLE DRAINAGE MEASURES OF BOTH SOURCE CONTROL AND ATTENUATION TO ACHIEVE THE REMAINING SITE DISCHARGE AND STORAGE NEEDS. THE BELOW SUDS FEATURES ARE RECOMMENDED AS PART OF THE ON PLOT AND SITE WIDE DRAINAGE:

- DETENTION BASINS
- BELOW GROUND TANKS
- MULGA ATTENUATION
- SWALES
- FILTER TRENCHES
- BLUE/ GREEN ROOFS
- RAIN GARDENS
- OVERSIZED PIPES



**OUTFALL 5:**  
PROPOSED DISCHARGE RATE TO BE LIMITED TO  $Q_{bar}$ . THE OVERALL SITE DISCHARGE RATE HAS PRO-RATA SPLIT TO DETERMINE THE CONTRIBUTING CATCHMENT AREAS AND ASSOCIATED DISCHARGE RATES:

- CATCHMENT AREA = 8.7ha
- 45.34L/s FOR ALL STORM EVENTS THE 1 IN 100 YEAR EVENTS (INCL. 40% CC.)
- INITIAL STORAGE ESTIMATION = 7,850m<sup>3</sup> (OF WHICH 8,460m<sup>3</sup> PROVIDED BY DETENTION BASINS SHOWN) ACHIEVABLE ONLY WITH PLOTS BEING PUMPED INTO BASIN.

THIS WILL BE ACHIEVED VIA A VORTEX FLOW CONTROL DEVICE BEFORE DISCHARGING INTO THE WATERCOURSE VIA A HEADWALL.

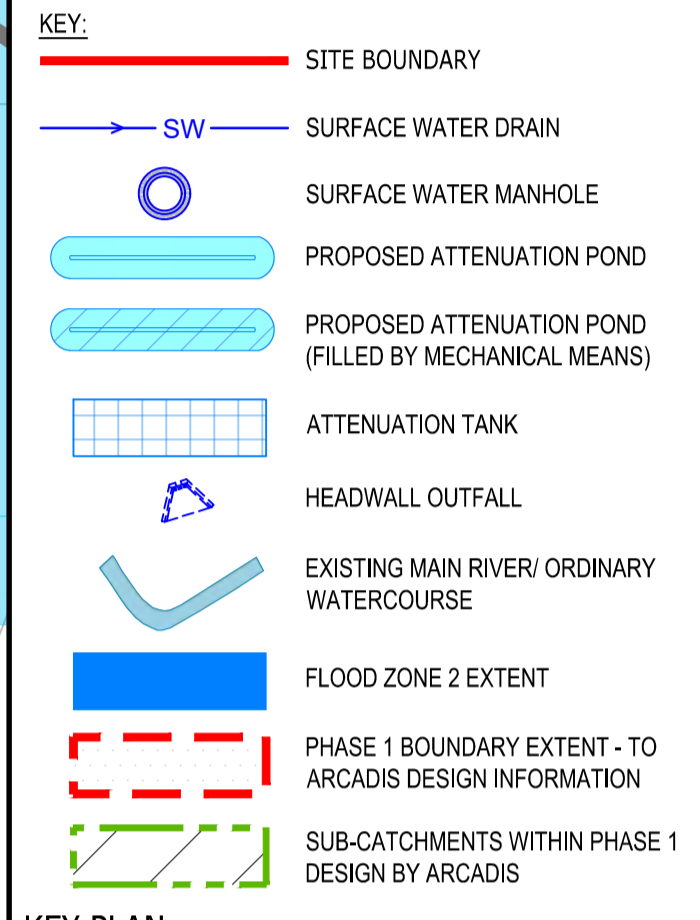
**GENERAL DESIGN NOTES:**

- SURFACE WATER DRAIN POINTS CONNECTING INTO SITE WIDE NETWORK SUBJECT TO FURTHER CONFIRMATION AND DETAILED DESIGN DEVELOPMENT.
- PROPOSED ONSITE UTILITY SERVICES ARE LIMITED DUE TO THE GREENFIELD NATURE OF THE SITE. COORDINATION WITH EXISTING AND PROPOSED SERVICES TO BE UNDERTAKEN DURING DETAILED DESIGN.
- THE DRAINAGE STRATEGY IS SUBJECT TO WEST SUSSEX COUNTY COUNCIL 'LFA' AND ENVIRONMENT AGENCY APPROVAL PRIOR TO CONSTRUCTION.
- FURTHER COORDINATION REQUIRED WITH FINAL TREE LOCATIONS AND DETAILS TO AVOID CLASHES WITH BELOW GROUND DRAINAGE.
- SURFACE WATER DRAINAGE DESIGN HAS BEEN DEVELOPED BASED ON EXISTING SITE TOPOGRAPHY. THE SURFACE WATER DRAINAGE STRATEGY IS TO BE FURTHER REVIEWED AND COORDINATED AS THE SITE LEVEL DESIGN DEVELOPS.

ADDITIONAL TANK THIS WILL BE A DEVICE BEFORE A HEADWALL.

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- Notes
- DO NOT SCALE FROM THIS DRAWING.
  - ALL DIMENSIONS ARE MILLIMETRES U.N.O.
  - THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL RELEVANT ARCHITECTS AND ENGINEERS DRAWINGS AND SPECIFICATIONS.
  - ALL DRAINAGE WORKS TO BE CONSTRUCTED IN ACCORDANCE WITH 'SEWERAGE SECTOR GUIDANCE APPENDIX C (FORMERLY KNOWN AS 'SEWERS FOR ADOPTION'), BUILDING REGULATIONS PART H AND CIVIL ENGINEERING SPECIFICATION FOR THE WATER INDUSTRY WHERE APPROPRIATE.
  - DRAWINGS HAVE BEEN PRODUCED BASED ON:
    - PRIOR & PARTNERS - MASTERPLAN LAYOUT - 230208\_WO1\_SITE A DETAILED STUDIES
    - OS BASE MAP



| Rev | Description               | Date       | By     | App |
|-----|---------------------------|------------|--------|-----|
| P05 | UPDATED RED LINE BOUNDARY | 10.06 2025 | GG PMG | SP  |
| P04 | DRAFT ISSUE               | 16.04 2025 | GG PMG | SP  |
| P03 | DRAFT ISSUE               | 05.03 2025 | GG PMG | DS  |
| P02 | DRAFT ISSUE               | 10.10 2023 | PMG MS | DS  |
| P01 | DRAFT ISSUE               | 25.05 2023 | PMG MS | DS  |

**STAGE 2**

**WEST OF IFIELD**

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**SITE WIDE SURFACE WATER DRAINAGE GENERAL ARRANGEMENT SHEET 1**

|                     |              |        |          |
|---------------------|--------------|--------|----------|
| Project No:         | Scale (B41): | Drawn: | Date:    |
| 1620007949-001      | 1:2000       | PMG    | MAY 2023 |
| Drawing No:         | Rev:         |        |          |
| RAM-XX-XX-DR-C-0100 | P05          |        |          |

**SURFACE WATER DRAINAGE STRATEGY:**

THE SITE BOUNDARY ACCOUNTS FOR 203ha HOWEVER, OF THE EXISTING SITE, IT IS CONSIDERED THAT THE DEVELOPED AREA CONTRIBUTING TO CHANGES IN THE DRAINING CHARACTERISTICS OF THE EXISTING GREENFIELD SITE ACCOUNTS FOR A TOTAL AREA OF 107ha. EXISTING WATERCOURSE DETAILS, LOCATION AND LEVELS TO BE CONFIRMED BY FURTHER SITE INVESTIGATIONS TO AID WITH DESIGN FOR DISCHARGE CONNECTIONS. NO DITCHES OR WATERCOURSES HAVE BEEN IDENTIFIED AS REQUIRING DIVERSION AS PART OF THE DEVELOPMENT PROPOSALS.

THE PROPOSED SITE WIDE DRAINAGE STRATEGY WILL COMPLY WITH THE BELOW INCLUSIONS FOR ONSITE SURFACE WATER MANAGEMENT AND MITIGATE FLOOD RISK OFF SITE:

- PROPOSED DISCHARGE RATE IS TO RETAIN GREENFIELD RUNOFF RATES TO BE RESTRICTED TO  $Q_{bar} = 300.38 \text{ L/S}$
- INITIAL STORAGE VOLUME REQUIRED FOR THE MAIN SITE TO MANAGE STORM EVENTS UP TO AND INCLUDING THE 1:100 YEARS STORM EVENT WITH THE INCLUSION FOR 40% CLIMATE CHANGE IS ESTIMATED TO BE =  $52,113\text{m}^3$
- ADD: THE TOTAL STORAGE ACCOMMODATED AS PART OF THE SITE WIDE STRATEGY THROUGH DETENTION BASINS/ BELOW GROUND STORAGE TANKS IS =  $36,985\text{m}^3$

PROPOSED SURFACE WATER DRAINAGE STRATEGY IS BASED ON DISCHARGE RATE OF RESTRICTING EXISTING GREEN FIELD RUNOFF TO  $Q_{bar}$  FOR THE PROPOSED DEVELOPMENT THROUGH THE USE OF ATTENUATION. IT IS NOT ANTICIPATED THAT THE STORAGE VOLUME IDENTIFIED ABOVE WOULD BE SUFFICIENT ON ITS OWN TO BE MANAGE WITHIN THE OVERALL SITE WIDE STORAGE AS SHOWN. IT WILL THEREFORE BE NECESSARY FOR THE REMAINING ATTENUATION TO BE PROVIDED WITHIN THE DEVELOPMENT PLOTS VIA SUSTAINABLE DRAINAGE MEASURES OF BOTH SOURCE CONTROL AND ATTENUATION TO ACHIEVE THE REMAINING SITE DISCHARGE AND STORAGE NEEDS. THE BELOW SUDS FEATURES ARE RECOMMENDED AS PART OF THE ON PLOT AND SITE WIDE DRAINAGE:

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- BELOW GROUND TANKS
- MUGA ATTENUATION
- SWALES
- FILTER TRENCHES
- BLUE/ GREEN ROOFS
- RAIN GARDENS
- OVERSIZED PIPES

**OUTFALL 2:**  
PROPOSED DISCHARGE RATE TO BE LIMITED TO  $Q_{bar}$ . THE OVERALL SITE DISCHARGE RATE HAS PRO-RATA SPLIT TO DETERMINE THE CONTRIBUTING CATCHMENT AREAS AND ASSOCIATED DISCHARGE RATES:  
- CATCHMENT AREA = 11.1ha  
- 57.72l/s FOR ALL STORM EVENTS THE 1 IN 100 YEAR EVENTS (INCL. 40% CC.)  
- INITIAL STORAGE ESTIMATION =  $10,000\text{m}^3$  (OF WHICH  $4,835\text{m}^3$  PROVIDED BY DETENTION BASINS SHOWN)  
- ADDITIONAL  $5,165\text{m}^3$  PROVIDED BY ATTENUATION TANK  
THIS WILL BE ACHIEVED VIA A VORTEX FLOW CONTROL DEVICE BEFORE DISCHARGING INTO THE WATERCOURSE VIA A HEADWALL.

**OUTFALL 1:**  
PROPOSED DISCHARGE RATE TO BE LIMITED TO  $Q_{bar}$ . THE OVERALL SITE DISCHARGE RATE HAS PRO-RATA SPLIT TO DETERMINE THE CONTRIBUTING CATCHMENT AREAS AND ASSOCIATED DISCHARGE RATES:  
- CATCHMENT AREA = 11.94ha  
- 62.1l/s FOR ALL STORM EVENTS THE 1 IN 100 YEAR EVENTS (INCL. 40% CC.)  
- INITIAL STORAGE ESTIMATION =  $10,755\text{m}^3$  (OF WHICH  $395\text{m}^3$  PROVIDED BY DETENTION BASINS SHOWN)  
THIS WILL BE ACHIEVED VIA A VORTEX FLOW CONTROL DEVICE BEFORE DISCHARGING INTO THE WATERCOURSE VIA A HEADWALL.

**OUTFALL 3:**  
PROPOSED DISCHARGE RATE TO BE LIMITED TO  $Q_{bar}$ . THE OVERALL SITE DISCHARGE RATE HAS PRO-RATA SPLIT TO DETERMINE THE CONTRIBUTING CATCHMENT AREAS AND ASSOCIATED DISCHARGE RATES:  
- CATCHMENT AREA = 19.3ha  
- 100.36l/s FOR ALL STORM EVENTS THE 1 IN 100 YEAR EVENTS (INCL. 40% CC.)  
- INITIAL STORAGE ESTIMATION =  $17,383\text{m}^3$  (OF WHICH  $7,255\text{m}^3$  PROVIDED BY DETENTION BASINS SHOWN)  
- ADDITIONAL  $8,200\text{m}^3$  PROVIDED BY ATTENUATION TANK  
THIS WILL BE ACHIEVED VIA A VORTEX FLOW CONTROL DEVICE BEFORE DISCHARGING INTO THE WATERCOURSE VIA A HEADWALL.

**OUTFALL 4:**  
PROPOSED DISCHARGE RATE TO BE LIMITED TO  $Q_{bar}$ . THE OVERALL SITE DISCHARGE RATE HAS PRO-RATA SPLIT TO DETERMINE THE CONTRIBUTING CATCHMENT AREAS AND ASSOCIATED DISCHARGE RATES:  
- CATCHMENT AREA = 6.8ha  
- 35.36l/s FOR ALL STORM EVENTS THE 1 IN 100 YEAR EVENTS (INCL. 40% CC.)  
- INITIAL STORAGE ESTIMATION =  $6,125\text{m}^3$  (OF WHICH  $1,450\text{m}^3$  PROVIDED BY DETENTION BASINS SHOWN)  
- ADDITIONAL  $1,225\text{m}^3$  PROVIDED BY ATTENUATION TANK  
THIS WILL BE ACHIEVED VIA A VORTEX FLOW CONTROL DEVICE BEFORE DISCHARGING INTO THE WATERCOURSE VIA A HEADWALL.

**GENERAL DESIGN NOTES:**

- SURFACE WATER DRAIN POINTS CONNECTING INTO SITE WIDE NETWORK SUBJECT TO FURTHER CONFIRMATION AND DETAILED DESIGN DEVELOPMENT.
- PROPOSED ONSITE UTILITY SERVICES ARE LIMITED DUE TO THE GREENFIELD NATURE OF THE SITE. COORDINATION WITH EXISTING AND PROPOSED SERVICES TO BE UNDERTAKEN DURING DETAILED DESIGN.
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- FURTHER COORDINATION REQUIRED WITH FINAL TREE LOCATIONS AND DETAILS TO AVOID CLASHES WITH BELOW GROUND DRAINAGE.
- SURFACE WATER DRAINAGE DESIGN HAS BEEN DEVELOPED BASED ON EXISTING SITE TOPOGRAPHY. THE SURFACE WATER DRAINAGE STRATEGY IS TO BE FURTHER REVIEWED AND COORDINATED AS THE SITE LEVEL DESIGN DEVELOPS.

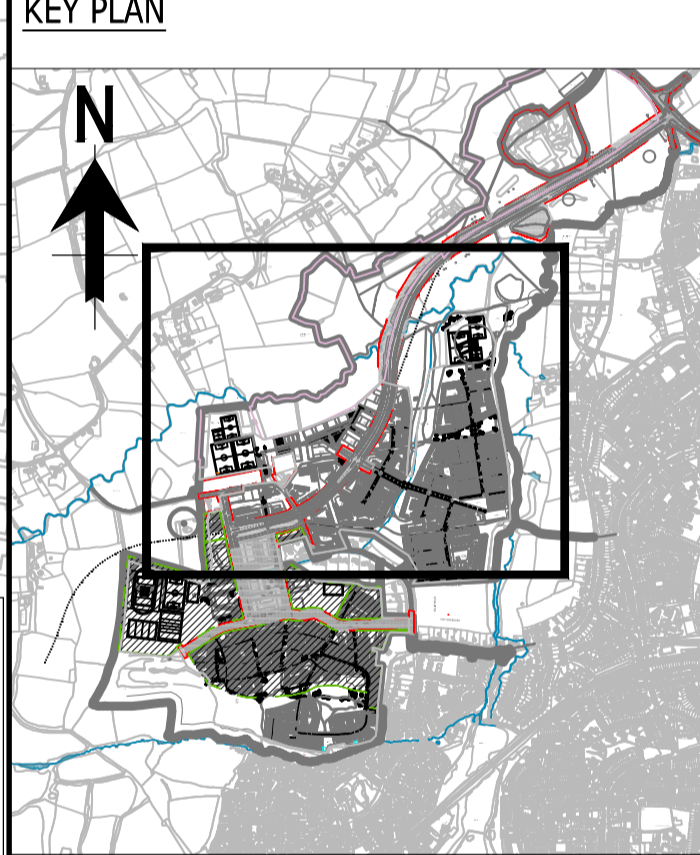


FOR CONTINUATION SEE DRAWING RAM-XX-XX-DR-C-0100

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  - DRAWINGS HAVE BEEN PRODUCED BASED ON:
    - PRIOR & PARTNERS - MASTERPLAN LAYOUT - 230208\_WOJ\_SITE A DETAILED STUDIES
    - OS BASE MAP

- KEY:
- SITE BOUNDARY
  - SURFACE WATER DRAIN
  - SURFACE WATER MANHOLE
  - PROPOSED ATTENUATION POND
  - PROPOSED ATTENUATION POND (FILLED BY MECHANICAL MEANS)
  - ATTENUATION TANK
  - HEADWALL OUTFALL
  - EXISTING MAIN RIVER/ ORDINARY WATERCOURSE
  - FLOOD ZONE 2 EXTENT
  - PHASE 1 BOUNDARY EXTENT - TO ARCADIS DESIGN INFORMATION
  - SUB-CATCHMENTS WITHIN PHASE 1 DESIGN BY ARCADIS



| Rev | Description               | Date       | By     | App |
|-----|---------------------------|------------|--------|-----|
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| P01 | DRAFT ISSUE               | 25.05 2023 | PMG MS | DS  |

**STAGE 2**

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**SITE WIDE SURFACE WATER DRAINAGE GENERAL ARRANGEMENT SHEET 2**

|                     |              |        |          |
|---------------------|--------------|--------|----------|
| Project No:         | Scale (B41): | Drawn: | Date:    |
| 1620007949-001      | 1:2000       | PMG    | MAY 2023 |
| Drawing No:         | Rev:         |        |          |
| RAM-XX-XX-DR-C-0101 | P05          |        |          |

## Appendix 6 Foul Water Connection point