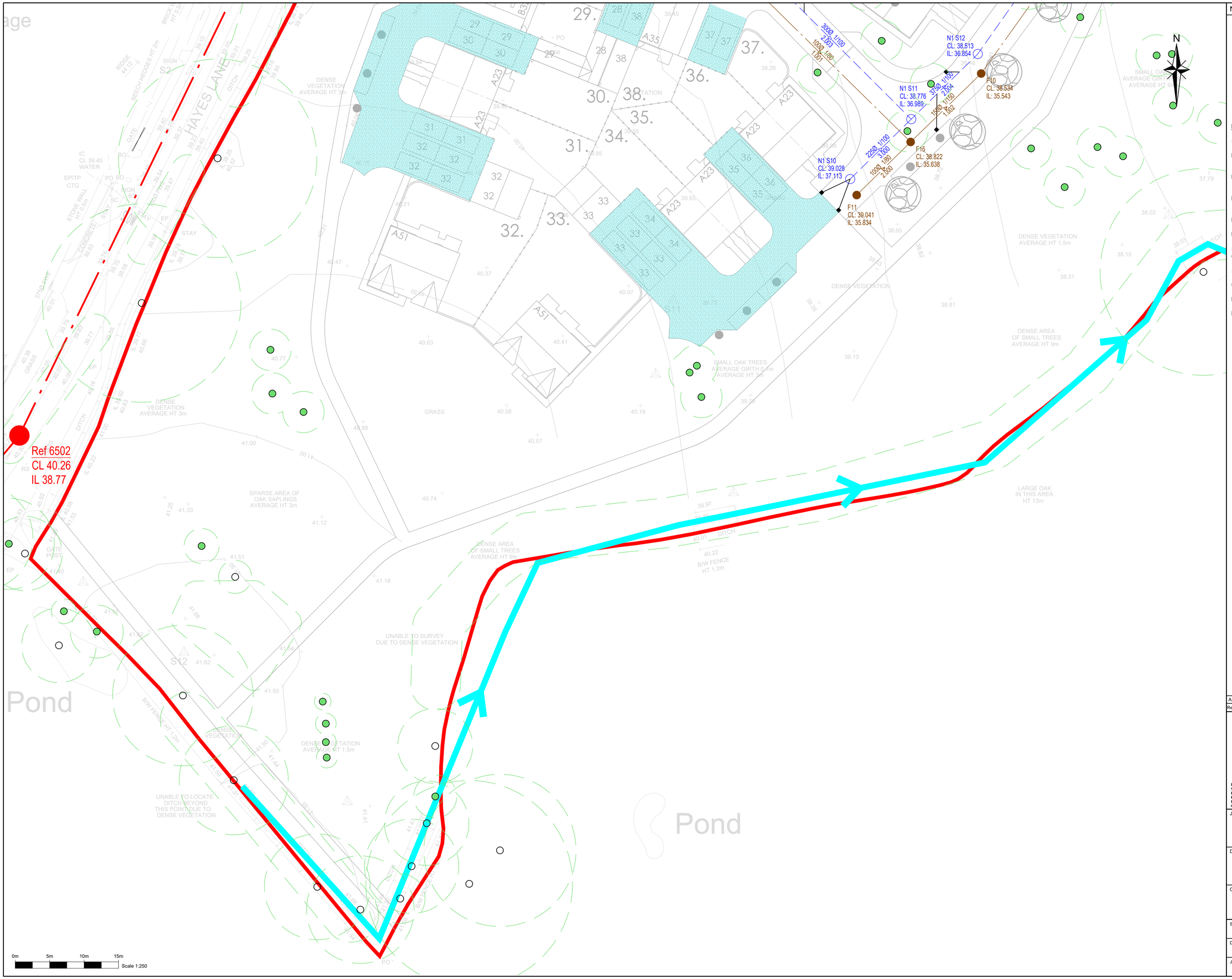
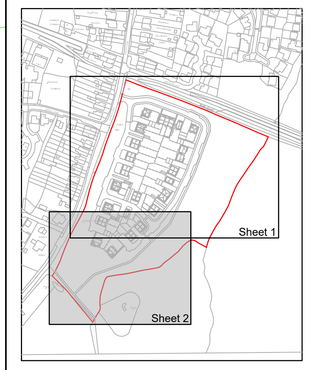


NOTES

1. Do not scale from this drawing.
2. All dimensions are in metres unless stated otherwise.
3. Topographical survey has been provided by others. C&A accept no responsibility for any inaccuracies.
4. Flood extent is based on flood map for planning, adjusted to reflect detailed levels from topographical survey (refer to FRA for further details)

Key

- Site Boundary
- Proposed Surface Water Network
- Proposed Foul Water Network
- Proposed Headwall
- Proposed Existing Foul Drainage
- Existing Ditch
- Surface Water Flooding Extent (1:100)
- Attenuation Tanks
- 6m Drainage Easement
- Permeable Paving
- Hydrobrake Flow Control



Rev	Amendments	Drn	Chk	App	Date
A	Site layout updated.	TB			Aug 25

Charles & Associates

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East Malling Trust Estate
Bradbourne Lane
Aylesford
Kent
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01732 448120

Job Title
Hayes Lane,
Slinfold

Drawing Title
Preliminary Drainage Strategy
Sheet 2

Client
TILCo Limited

Scale	Date	Designed
1:250	June 25	TB

Drawn	Checked	Approved
TB	GAC	GAC

Job No	Drawing No	Rev
22-011	22-011-008	A

N

NOTE:
VARYING PIPE SIZES RECORDED BETWEEN IC1, IC2 & IC3. IF CRITICAL, CCTV SURVEY RECOMMENDED TO CONFIRM.

0 5 10 20 40 50
1:500

NOTES

1. Do not scale from this drawing.
2. All dimensions are in metres unless stated otherwise.
3. Topographical survey has been provided by others. C&A accept no responsibility for any inaccuracies.

Key

Site Boundary

Dwelling Roof Catchments +10% (Urban Creep) — xx

Other Roof Catchments — xx

Road Catchments — xx

Rev	Amendments	Dm	Chk	App	Date
A	Site layout updated.	TB			Aug 25

Charles & Associates

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Job Title
Hayes Lane,
Slinfold

Drawing Title
Impermeable Areas Plan

Client
TILCo Limited

Scale	Date	Designed
1:500	June 25	CD

Drawn	Checked	Approved
CD	TB	GAC

Job No	Drawing No	Rev
22-011	22-011-012	A

Appendix G Southern Water Capacity Check

from Southern Water

Thomas Butler
Charles & Associates Consulting Engineers Ltd
Landmark House, Station Road
Hook
Hampshire
RG27 9HA

Your ref
20913

Our ref
DSA000044516

Date
16 June 2025

Contact
Tel 0330 303 0119

Dear Mr Butler,

Level 1 Capacity Check Enquiry: Hayes Lane, Slinfold, RH13 0BH.

We have completed the capacity check for the above development site and the results are as follows:

Foul Water

The enquiry has been reassessed to determine the capacity available for 0.32 l/s at manhole reference TQ11307702, Grid Reference: 511765 , 130799.

There is currently adequate capacity in the local sewerage network to accommodate a foul flow of 0.32 l/s for the above development at manhole reference TQ11307702. Southern Water will not permit a surface water connection into the foul network. Please refer to our surface water management policy.

Connecting to our network

It should be noted that this information is only a hydraulic assessment of the existing sewerage network and does not grant approval for a connection to the public sewerage system. A formal Sewer Connection (S106) application is required to be completed and approved by Southern Water Services. To make an application visit: developerservices.southernwater.co.uk

Please note the information provided above does not grant approval for any designs/drawings submitted for the capacity analysis. The results quoted above are only valid for 12 months from the date of issue of this letter. **Any revisions in flow rate or alternative point of connection will require a new Pre-Planning Application to be submitted.**

Please get in touch via the Get Connected customer dashboard if you have any queries.

Should it be necessary to contact us please quote our above reference number in all communications relating to this application by email at southernwaterplanning@southernwater.co.uk

Yours sincerely,

Future Growth Planning Team
Developer Services

southernwater.co.uk/developing-building/planning-your-development

Appendix H Exceedance Flow Routes Drawing

N

NOTES

1. Do not scale from this drawing.
2. All dimensions are in metres unless stated otherwise.
3. Topographical survey has been provided by others. C&A accept no responsibility for any inaccuracies.

Key

Site Boundary

Proposed Surface Water Network

Overland Flow Route

Existing Ditch

NOTE:
VARYING PIPE SIZES RECORDED BETWEEN IC1, IC2 & IC3. IF CRITICAL, CCTV SURVEY RECOMMENDED TO CONFIRM.

0m 5m 10m 15m
Scale 1:250

Rev	Amendments	Dm	Chk	App	Date
A	Site layout updated.	TB			Aug 25

Charles & Associates

Issued by

Landmark House
Station Road
Hook
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www.c-a.uk.com

Job Title
Hayes Lane,
Slinfold

Drawing Title
Overland Flow Routes

Client
TILCo Limited

Scale	Date	Designed
1:500	June 25	TB
Drawn	Checked	Approved
TB	GAC	GAC
Job No	Drawing No	Rev
22-011	22-011-011	A

Appendix I Maintenance Schedule

Surface Water Drainage SuDS Maintenance Schedule

Hayes Lane, Slinfold

22-011-005 Rev -

June 2025

Charles & Associates

Document Control Sheet

Project Name:	Hayes Lane, Slinfold
Project Number:	22-011
Report Title:	Surface Water Drainage SuDS Maintenance Schedule
Report Number:	005

Rev	Issue Purpose	Author	Checked	Reviewed	Approved	Date

C&A Consulting Engineers

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Contents

1 Introduction & Overview

1.1 Introduction

1.1.1 This document details the inspection and maintenance specification for the surface water drainage system including sustainable drainage systems (SuDS) and features utilised for the Hayes Lane, Slinfold.

1.1.2 This report has been prepared to submit to the Lead Local Flood Authority (West Sussex County Council (WSCC)) and the Local Planning Authority (Horsham District Council (HDC)) as part of a detailed planning application.

1.1.3 This report should be read in conjunction with drawings 22-011-007, 22-011-008 and the Flood Risk Assessment.

1.1.4 In addition, it will be made available to the adopting body and/or maintenance organisation that will ultimately control the SuDS assets.

1.1.5 Most of the piped surface water drainage system on the site will be offered to Southern Water under a Section 104 agreement of the Water Industry Act 1991. Once the system has been vested by Southern Water, they will become responsible for the maintenance of the adopted features of the surface water network including chambers, sewers, storage tanks and flow controls.

1.1.6 This document relates to surface water drainage features that will not be adopted by Southern Water. The features listed in this document are to be maintained by a suitably qualified management company.

1.1.7 Private drainage relating to individual properties such as gutters, rainwater down pipes, private inspection chambers, rodding eyes, linear channels, yard gullies and domestic soakaways and individual permeably paved drives are to be maintained by the property owner.

1.1.8 Maintenance requirements should be reviewed and adjusted as necessary following detailed design, specification and product choice of the drainage components.

1.2 Overview

1.2.1 This document is laid out in specific sections pertinent to SuDS type detailing:

- A description of the SuDS component and its use
- Maintenance requirements and frequencies
- Inspection requirements and frequencies

1.2.2 The activities listed are generic to SuDS types and represent the minimum maintenance and inspection requirements. However additional tasks, or varied maintenance frequency may be introduced as required. This may include:

- Measurement and recording of sediment levels
- Photographic recording of problem areas
- Increased frequency of litter removal in areas identified as litter hotspots
- Adjusted timescales of maintenance activities to minimise impact on wildlife (e.g. great crested newts, birds, voles, etc)
- The use of manual methods of maintenance to limit impact on wildlife habitat (e.g. management of emergent vegetation in ponds)

This specification has been based upon CIRIAs latest technical information for SuDS (C753 The SUDS Manual, CIRIA, London: 2015)

2 Permeable Paving

2.1 Description

2.1.1 Permeable paving provides a pavement suitable for pedestrian and vehicular traffic, while allowing rainwater to permeate through the surface and into the underlying sub-base. The water is temporarily stored in the sub-base before discharging to the underlying ground (infiltrating) or discharging to a piped network (tanked).

2.1.2 Permeable paving, together with associated substructures, are an efficient means of managing surface water runoff close to source – intercepting runoff, reducing the volume and frequency of runoff and providing a treatment medium. Treatment processes that occur within the surface structure, the subsurface matrix (including soil layers where infiltration can occur) and the geotextile layers include:

- Filtration
- Adsorption
- Biodegradation
- sedimentation

2.2 Inspection, Maintenance Activities and Frequency

2.2.1 Refer to Table 2.2.1 below for maintenance activities and frequency for permeable paving.

Table 2.2.1: Permeable Paving Inspection, Maintenance Activities and Frequency

Maintenance Category	Maintenance Activity	Frequency	Comments
Regular Maintenance	Brushing and vacuuming (standard cosmetic sweep over whole surface)	Annually (after autumn leaf fall) or reduced frequency, as required, based on site observations of clogging or manufacturer's recommendations.	Particular attention should be paid to areas where water runs onto permeable paving from adjacent impermeable areas as this area is most likely to collect the most sediment
Occasional Maintenance	Stabilise and mow contributing and adjacent areas	As required	
	Removal of weeds	As required – once per year on less frequently used pavements	

Maintenance Category	Maintenance Activity	Frequency	Comments
Remedial Actions	Remediate any landscaping which, through vegetation maintenance or soil slip, has been raised to within 50mm of the paving level	As required	
	Remedial work to any depressions, rutting or cracked/broken blocks considered detrimental to the structural performance or a hazard to users, and replace lost jointing material	As required	
	Rehabilitation of surface and upper substructure by remedial sweeping	10 – 15 years	Or as required if infiltration performance is reduced due to significant clogging
Monitoring	Initial inspection	Monthly	For minimum 3 months after installation. If required take remedial action
	Inspect for evidence of poor operation and weed growth	Quarterly, 48 hours after large storms for first 6 months	
	Inspect silt accumulation rates	Annually	
	Monitor inspection chambers	Annually	

3 Geocellular / Modular Systems

3.1 Description

3.1.1 Modular plastic geocellular systems with a high void ratio that can be used to create a below ground infiltration (soakaway) or storage structure.

3.2 Inspection, Maintenance Activities and Frequency

3.2.1 Refer to Table 4.2.1 below maintenance activities and frequency for geocellular systems.

Table 4.2.1: Geocellular System Inspection, Maintenance Activities and Frequency

Maintenance Category	Maintenance Activity	Frequency	Comments
Regular Maintenance	Inspect and identify any areas that are not operating correctly,	Monthly for 3 months, then 6 monthly	If required, take remedial action.
	Debris removal from catchment surface (where may cause risk to performance)	Monthly	
	Where rainfall infiltrates into blocks from above, check surface of filter for blockage by silt, algae or other matter.	Monthly (and after large storms)	Remove and replace surface infiltration medium as necessary
	Remove sediment from pre-treatment structures	Annually or as required	
Remedial Actions	Repair/rehabilitation of inlets, outlet, overflows and vents	As required	
Monitoring	Inspect / check all inlets, outlets, vents and overflows to ensure that they are in good condition and operating as designed	Annually and after large storms	

4 Attenuation Basin / Detention Basin

4.1 Description

4.1.1 Attenuation basins are surface storage basins or facilities that provide flow control through attenuation of stormwater runoff. They also facilitate some settling of particulate pollutants. Attenuation basins are normally dry and in certain situations the land may also function as a recreational facility.

4.2 Inspection, Maintenance Activities and Frequency

4.2.1 Refer to Table 5.2.1 below maintenance activities and frequency for attenuation basins.

Table 4.2.1: Attenuation Basin Inspection, Maintenance Activities and Frequency

Maintenance Category	Maintenance Activity	Frequency	Comments
Regular Maintenance	Litter and debris removal	Monthly	
	Grass cutting - for spillways and access routes	Monthly (during growing season), or as required	
	Grass cutting – meadow grass in and around basin	Half yearly (spring before nesting season and autumn)	
	Manage other vegetation and remove nuisance plants	Monthly (at start, then as required)	
	Tidy all dead growth before start of growing season	Annually	
	Remove sediment from inlets, outlet and forebay	Annually (or as required)	
	Manage wetland plants in outlet pool – where provided	Annually	
Occasional Maintenance	Re-seed areas of poor vegetation growth	Annually (or as required)	
	Prune and trim trees and remove cuttings	2 years (or as required)	
	Remove sediment from forebay, when 50% full and from micropools if volume reduced by >25%	3-10 years (or as required)	
Remedial Actions	Repair of erosion or other damage by re-seeding or re-turfing	As required	

Maintenance Category	Maintenance Activity	Frequency	Comments
	Repair/rehabilitation of inlets, outlets and overflows	As required	
	Re-level uneven surfaces and reinstate design levels	As required	
Monitoring	Inspect / check all inlets, outlets and overflows for blockage and clear if required	Monthly and after large storms	
	Inspect banksides, structures, pipework etc for evidence of physical damage	Monthly and after large storms	
	Inspect inlets and facility surface for silt accumulation. Establish appropriate silt removal frequencies	Half yearly	
	Check penstocks and other mechanical devices	Half yearly	
	Inspect inlets and pre-treatment components for silt accumulation.	Half yearly	Includes visual inspection of chamber (where present) & inspection of flow control.
	Check mechanical devices within chambers.	Half yearly	Includes cracking of seals on penstocks, testing Hydro- Brake by-pass door.

5 Oil Interceptor

5.1 Inspection, Maintenance Activities and Frequency

5.1.1 Maintenance of the oil interceptor will be required based on manufacturer's recommendations. Details will be confirmed following detailed specification. This will include periodic removal of silts and replacement of filter elements (three, six or twelve-monthly intervals depending on site conditions).

Appendix J Surface Water Drainage Statement

Surface Water Drainage Statement

In order to provide the required information on surface water drainage from the proposed development this pro-forma must be completed in full and be submitted with any planning application which seeks permission for 'major' development. This information contained in this form will be used by West Sussex County Council in its role as Lead Local Flood Authority and 'statutory consultee' on SuDs for all 'major' planning applications. The pro-forma is supported by the Defra/EA Guidance on Rainfall Runoff Management and can be completed using freely available tools including SuDS Tools. The pro-forma should be considered alongside other supporting SuDS Guidance, but focuses on ensuring flood risk is not made worse elsewhere. The SuDS solution must operate effectively for as long as the development exists. This pro-forma is based upon current industry standard practice.

1. Site Details

Site	Hayes Lane, Slinfold
Address & post code or LPA reference	Hayes Lane, Slinfold, Horsham, West Sussex, England
Grid reference	TQ 11801 30715
Is the existing site developed or Greenfield?	Greenfield
Total Site Area served by drainage system (excluding open space) (Ha)*	0.9 (excludes gardens)
Topographical survey plan showing existing site layout, site levels and drainage system	Yes

***** The Greenfield runoff off rate from the development which is to be used for assessing the requirements for limiting discharge flow rates and attenuation storage from a site should be calculated for the area that forms the drainage network for the site whatever size of site and type of drainage technique. Please refer to the Rainfall Runoff Management document or CIRIA manual for detail on this.

2. Impermeable Area

	Existing	Proposed	Difference (Proposed-Existing)	Notes for developers & Local Authorities
Impermeable area (ha) (areas to be shown on a plan)	0	0.9	0.9	If the proposed amount of impermeable surface is greater, then runoff rates and volumes will increase. Section 6 must be filled in. If proposed permeability is equal or less than existing, then section 6 can be skipped & section 7 filled in.
Drainage Method (infiltration/sewer/watercourse)	Watercourse	Watercourse	N/A	If different from the existing, please fill in section 3. If existing drainage is by infiltration and the proposed is not, discharge volumes may increase. Fill in section 6.

PPG Paragraph 080

3. Proposing to Discharge Surface Water via

	Yes	No	Evidence that this is possible	Notes for developers & Local Authorities
Existing and proposed micro-drainage calculations	X			Please provide micro-drainage calculations of existing and proposed run-off rates and volumes in accordance with a recognised methodology or the results of a full infiltration test (see line below) if infiltration is proposed.
Infiltration		X		e.g. soakage tests. Section 6 (infiltration) must be filled in if infiltration is proposed.
To watercourse	X			e.g. Is there a watercourse nearby? Please provide details of any watercourse to which the site drains including cross-sections of any adjacent water courses for appropriate distance upstream and downstream of the discharge point (as agreed with the LLFA and/or EA)
To surface water sewer		X		Confirmation from sewer provider that sufficient capacity exists for this connection.
Combination of above		X		e.g. part infiltration part discharge to sewer or watercourse. Provide evidence above.
Has the drainage proposal had regard to the SuDS hierarchy?	X			Evidence must be provided to demonstrate that the proposed Sustainable Drainage proposal has had regard to the SuDS hierarchy.
Layout plan showing where the sustainable drainage infrastructure will be located on site.	X			Please provide plan reference numbers showing the details of the site layout showing where the sustainable drainage infrastructure will be located on the site. If the development is to be constructed in phases this should be shown on a separate plan and confirmation should be provided that the sustainable drainage proposal for each phase can be constructed and can operate independently and is not reliant on any later phase of development.

Technical Standards S2 and S3

4. Peak Discharge Rates – This is the maximum flow rate at which surface water runoff leaves the site during a particular storm event.

	Existing Rates (l/s)	Proposed Rates (l/s)	Difference (l/s) (Proposed-Existing)	Notes for developers & Local Authorities
Greenfield QBAR	23.01 (Whole Site) 5.52 (0.9ha)	N/A	N/A	Mean annual Greenfield peak flow - QBAR is approx. 1 in 2 storm events. Use that figure in Section 7a.
1 in 1	19.56 (Whole Site) 4.69 (0.9ha)	5.4	0	Proposed discharge rates (with mitigation) should be no greater than existing rates for all corresponding storm events. e.g. discharging all flow from site at the existing 1 in 100 event increases flood risk during smaller events.
1 in 30	52.93 (Whole Site) 12.69 (0.9ha)	5.6	0	
1in 100	72.41 (Whole Site) 17.60 (0.9ha)	5.6	0	
1 in 100 plus climate change	N/A	5.9		To mitigate for climate change the proposed 1 in 100 +CC must be no greater than the existing 1 in 100 runoff rate. If not, flood risk increases under climate change should be added to the peak rainfall intensity. EA Guidance - Flood Risk Assessments: Climate Change Allowances (Published Feb 2016) https://www.gov.uk/guidance/flood-risk-assessments-climate-change-allowances

Technical Standards S4 to S9

5. Calculate discharge volumes –The total volume of water leaving the development site for a particular rainfall event. Introducing new impermeable surfaces increases surface water runoff and may increase flood risk outside the development.

	Existing Volume (m^3)	Proposed Volume (m^3)	Difference (m^3) (Proposed-Existing)	Notes for developers & Local Authorities
1 in 1	increase in volume due to impermeable area will be managed by limiting discharge to Qbar rates - see section 7a			Proposed discharge volumes (without mitigation) should be no greater than existing volumes for all corresponding storm events. Any increase in volume increases flood risk elsewhere. Where volumes are increased section 6 must be filled in.
1 in 30				
1in 100				

1 in 100 plus climate change		To mitigate for climate change the volume discharge from site must be no greater than the existing 1 in 100 storm event. If not, flood risk increases under climate change.

6. Calculate attenuation storage – In order to minimise the negative impact on flood risk resulting from increased volumes runoff from the proposed development, storage must be provided.

		Notes for developers & Local Authorities
Storage volume required to retain discharge rates as existing (m^3)	1155m3	Volume of water to attenuate on site if discharging at existing rates. Can't be used where discharge volumes are increasing
Where will the storage be provided on site?	Basin and attenuation tank as per drawings	

7. How is Storm Water stored on site?

Storage is required for the additional volume from site but also for holding back water to slow down the rate from the site. This is known as attenuation storage and long term storage. The intention is to not discharge that volume into the watercourses so as not to increase flood risk elsewhere.

			Notes for developers & Local Authorities
Infiltration	**State the Site's Geology/drift material overlaying)**	Clay	Avoid infiltrating in made ground.
	Does the site have a high ground water table? Yes/No?	No	If yes, please provide details of the site's hydrology.
	Is the site within a known Source Protection Zones (SPZ)? Yes/No?	No	Infiltration rates are highly variable and refer to Environment Agency website to identify and source protection zones (SPZ)
	Are infiltration rates suitable?	N/A	Infiltration rates should be no lower than $1x10^{-6}$ m/s.
	Is the site contaminated? If yes, consider advice from others on whether infiltration can happen.	N/A	Water should not be infiltrated through land that is contaminated. The Environment Agency may provide bespoke advice in planning consultations for contaminated sites that should be considered.
	State the distance between a proposed infiltration device base and the ground water (GW) level	N/A	Need 1m (min) between the base of the infiltration device & the water table to protect Groundwater quality & ensure GW doesn't enter infiltration devices. Avoid infiltration where this isn't possible.
	Were infiltration rates obtained by desk study or infiltration test?	N/A	Infiltration rates can be estimated from desk studies at most stages of the planning system if a back-up attenuation scheme is provided.
Is infiltration feasible?	**Yes/No?**	No	If infiltration is not feasible how will the additional volume be stored?. The applicant should then consider the following options in the next section.

7a. Storage requirements

Where infiltration is not possible, then the developer must confirm that either of the two options below will be implemented for dealing with the amount of water that needs to be stored on site.

Option 1 Simple – Store both the additional volume and attenuation volume in order to make a final discharge from site at **QBAR**. This is preferred if no infiltration can be made on site. This very simply satisfies the runoff rates and volume criteria.

Option 2 Complex – If some of the additional volume of water can be infiltrated back into the ground, the remainder can be discharged at a very low rate of 2 l/sec/hectare. A combined storage calculation using the partial permissible rate of 2 l/sec/hectare and the attenuation rate used to slow the runoff from site.

		Notes for developers & Local Authorities
Please confirm what option has been chosen and how much storage is required on site.	Option 1: 1155m3	The developer at this stage should understand the site characteristics and be able to explain what the storage requirements are on site and how it will be achieved.

8. Additional Consideration to comply with the Technical Standards and PPG

		Notes for developers & Local Authorities
Which Drainage Systems measures have been used?	Permeable Paving, Catchpit Manholes, Trapped Gullies, By-Pass Separator and Attenuation Basin.	SUDS can be adapted for most situations even where infiltration isn't feasible e.g. impermeable liners beneath some SUDS devices allows treatment but not infiltration. See CIRIA SUDS Manual C753.
How will exceedance events be catered on site without increasing flood risks (both on site and outside the development)?	Levels will direct exceedance flows along existing routes towards open space areas via highway corridors.	**Safely:** not causing property flooding or posing a hazard to site users i.e. no deeper than 300mm on roads/footpaths
How are rates being restricted?	Hydrobrakes	Hydrobrakes to be used where rates are between 2l/s to 5l/s. Orifices not be used below 5l/s as the pipes may block. Pipes with flows < 2l/s are prone to blockage.
Drainage during construction period		Provide details of how drainage will be managed during the construction period including any necessary connections, impacts, diversions and erosion control.
Key Drainage components / Features	Permeable Paving, Flow controls and Attenuation Basin and attenuation tanks	Which component if blocked (even partial) will lead to flooding?

Technical Standards S10 to S12

9. Management and Maintenance of SuDs

Details are required to be provided of the management and maintenance plan for the SUD, including for the individual plots in perpetuity.

How is the entire drainage system to be maintained in perpetuity?	See Management plan provided.	Clear details of the maintenance proposals of all elements of the proposed drainage system must be provided to show that all parts of SuDs are effective and robust. Provide a management plan to describe the SUDS scheme and set out the management objectives for the site. It should consider how the SuDs will perform and develop over time anticipating any additional maintenance tasks to ensure the system continues to perform as designed. — Specification notes that describe how work is to be undertaken and the materials to be used. — A maintenance schedule describes what work is to be done and when it is to be done using frequency and performance requirements as appropriate. — A site plan showing maintenance areas, control points and outfalls. Responsibility for the management and maintenance of each element of the SUDS scheme will also need to be detailed within the Management Plan. Where open water is involved please provide a health and safety plan within the management plan.
Please confirm the owners/adopters of the entire drainage systems throughout the development. Please list all the owners.	Individual property owners for on-plot drainage. Southern Water for piped drainage network elements and Client appointed management company for the remainder	If these are multiple owners then a drawing illustrating exactly what features will be within each owner's remit must be submitted with this Proforma. Please give details of each feature and how it will be managed in accordance with the details in the management plan.
Please provide details demonstrating that any third party agreements required using land outside the application site have been secured.	N/A	

The above form should be completed using evidence from information which should be appended to this form. The information being submitted should be proportionate to the site conditions, flood risks and magnitude of development. It should serve as a summary of the drainage proposals and should clearly show that the proposed discharge rate and volume as a result of development will not be increasing. Where there is an increase in discharge rate or volume, then the relevant section of this form must be completed with clear evidence demonstrating how the requirements will be met.

This form is completed using factual information and can be used as a summary of the surface water drainage strategy on this site.

Form Completed By……Tom Butler…….....

Qualification of person responsible for signing off this pro-forma .....EngTech MICE.....

Company……Charles & Associates……,.....

On behalf of (Client's details) .....The Incidental Land Company.....

Date:……June 2025.....